Earthquakes Risk Reduction in MNBC Provisions

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Building Engineering Institute Myanmar Earthquake Committee



Content

- · Earthquakes
- · Earthquake Engineering
- Earthquake Design in Myanmar National Building Code
- · Living with Earthquake





မျက်ဖြေ လင်္ကာ (အနန္တသူရိယ)

သူတည်းတစ်ယောက်၊ ကောင်းဖို့ရောက်မူ သူတစ်ယောက်မှာ၊ ပျက်လင့်ကာသာ ဓမ္မတာတည်း။

ရွှေအိမ်နန်းနှင့်၊ ကြနန်းလည်းခံ မတ်ပေါင်းရံလျက်၊ ပျော်စံရိမ်ငြိမ် စည်းစိမ်မကွာ၊ မင်းချမ်းသာကား သမုဒ္ဒရာ၊ ရေမျက်နှာထက် ခဏတက်သည့်၊ ရေပွက်ပမာ တစ်သက်လျာတည်း။



What is Our Earth anyway?

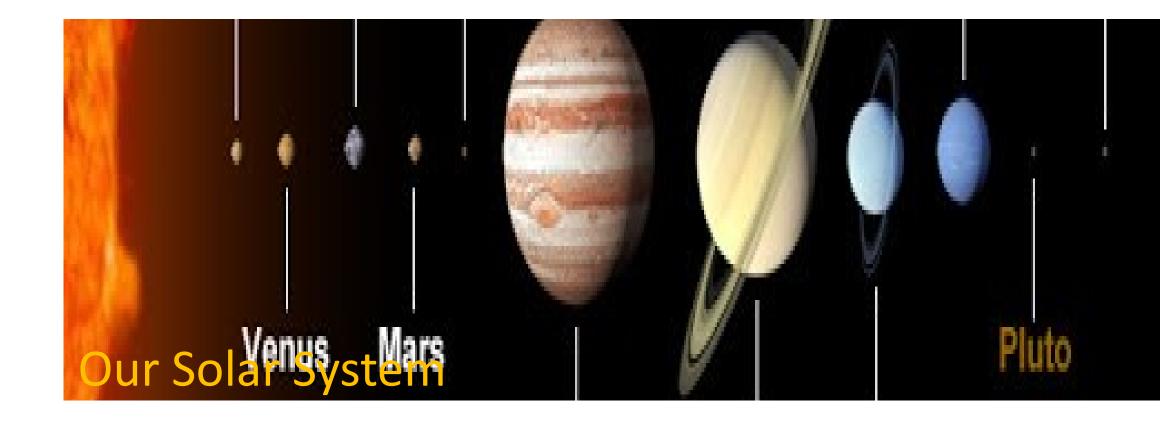
A tiny stone in space

Rotating itself daily

Circling around the SUN annually

Our Earth is NOT an STATIC object







Rotating itself daily

- 1 circle every day at about
 1000 km/hr, 600 mph
- about the speed of airplanes

Circling around the SUN

- 1 circle every year at 107,000 km/h, 67,000 mph
- 40 times faster than F15

Our Earth is NOT a STATIC object

5. McDonnell Douglas F-15 Eagle (1650 mph)



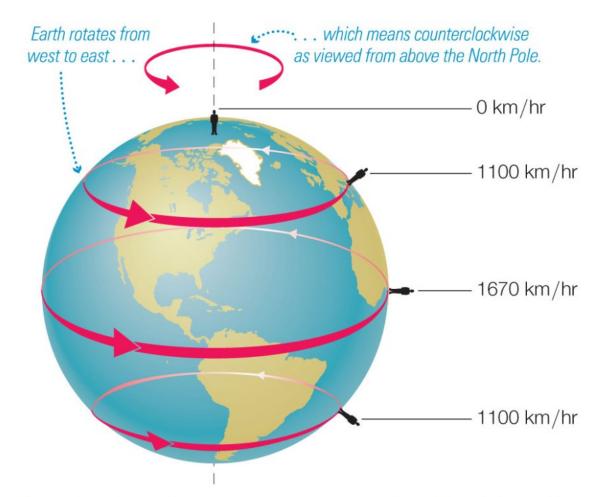


Figure 1.26 – As Earth rotates, your speed around Earth's axis depends on your location: The closer you are to the equator, the larger the path you move around each day, and therefore the faster you travel with Earth's rotation. *Credit: The Cosmic Perspective*

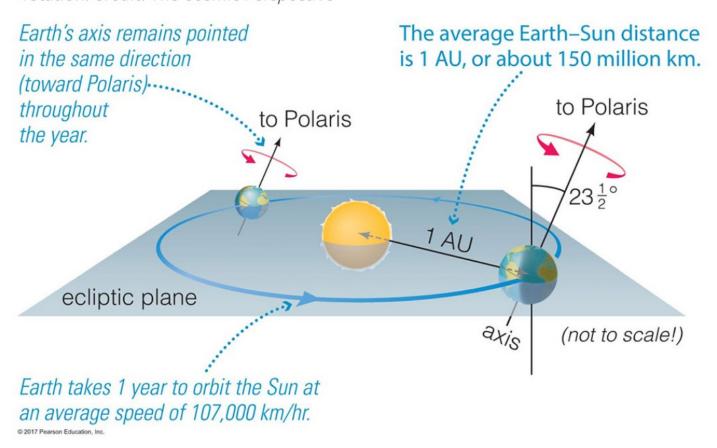


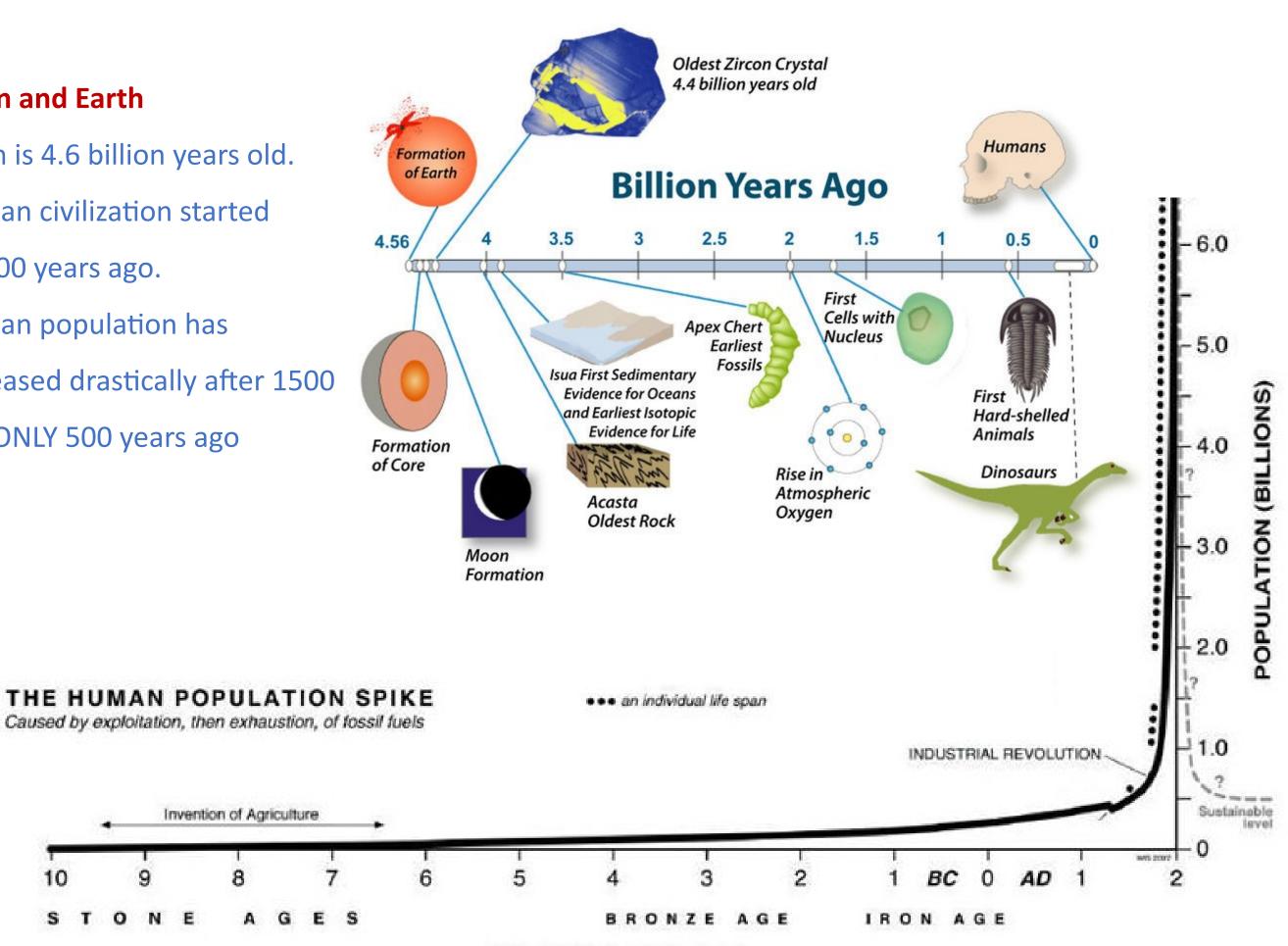
Figure 1.28 – This diagram shows key features of Earth's orbit around the Sun. *Credit: The Cosmic Perspective*



Human and Earth

- Earth is 4.6 billion years old.
- Human civilization started 10,000 years ago.
- Human population has increased drastically after 1500 AD, ONLY 500 years ago

Invention of Agriculture



THOUSANDS OF YEARS



Design Life

 Design life will vary according to the type and use of the element being considered. BS EN 1990, Eurocode - Basis of structural design, (Eurocode 0) gives indicative design lives for various types of structure:

Category 1:

Temporary structures, not including structures or parts of structures that can be dismantled with a view to being re-used – 10 years.

Category 2:

Replaceable structural parts, e.g. gantry girders, bearings – 10 to 25 years.

Category 3:

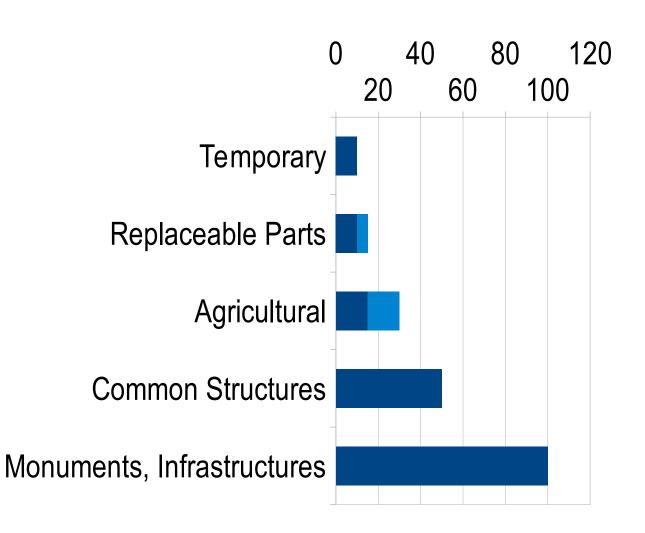
Agricultural and similar buildings – 15 to 30 years.

Category 4:

Building structures and other common structures – 50 years.

Category 5:

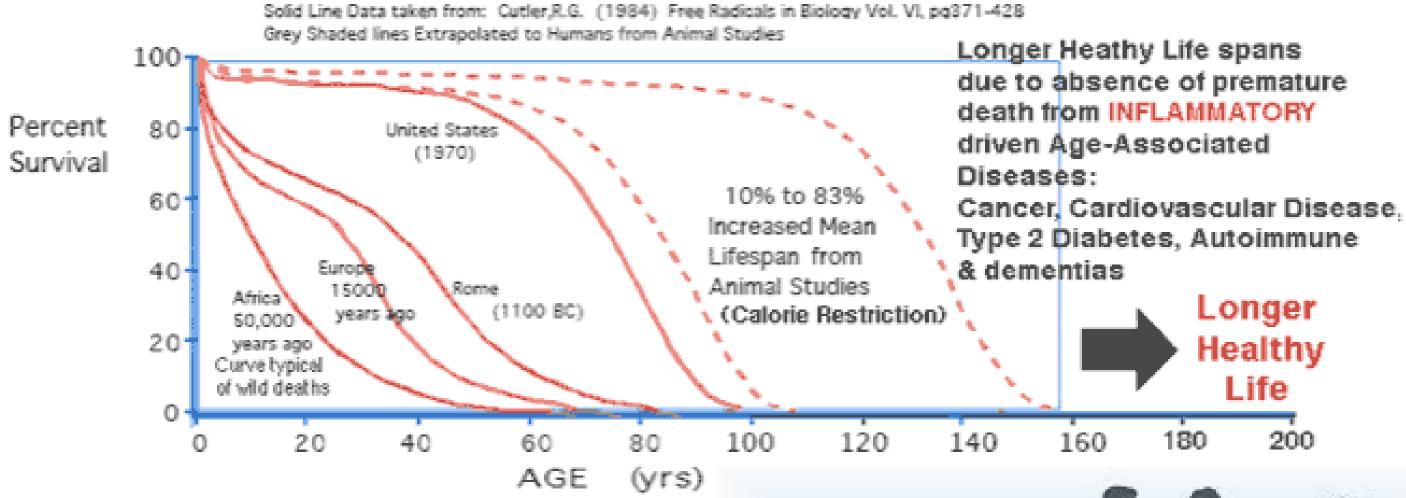
Monumental building structures, bridges and other civil engineering structures – 100 years.





50 Years of Human Windows on 4 Billions Years of Earth

2 SURVIVAL OF THE HUMAN POPULATION IN HISTORY



Nearly 50,000 years back people were generally dead by age 20





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because we are considering 50
years structural life span
window over 4.6 billion years
old planet



How the earth is made of?

Section Cut

- Crust uppermost thin layer,3-44 miles thick
- Upper Mantle 200-250 milesthick, 1600 F
- Lower Mantle 1800 milesthick, 7000 F
- Fluid Core 1,400 miles thick, 7000 F
- Solid Core 760 miles thick, 9800F

We are sitting on a boiling pot of LAVA

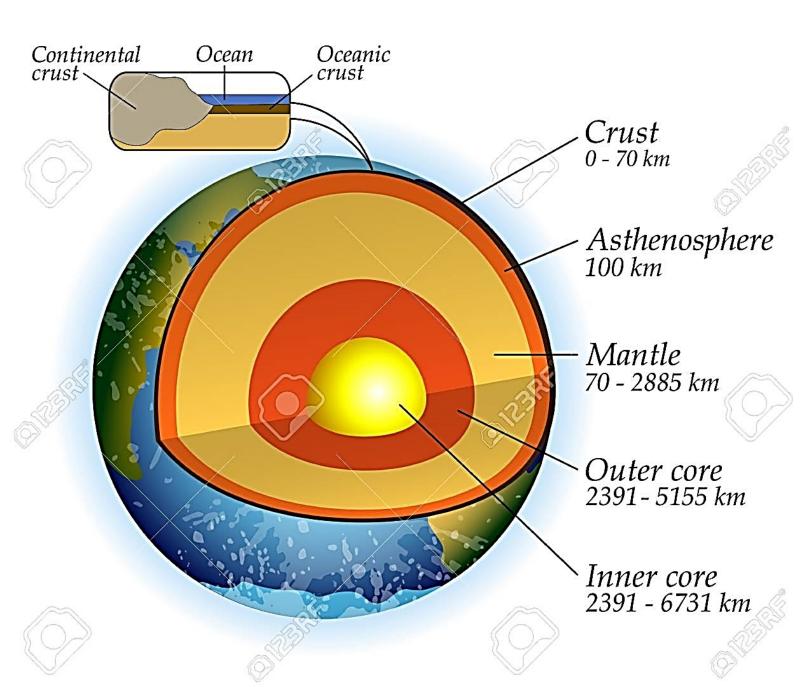




10 min

EARTH IN CROSS SECTION

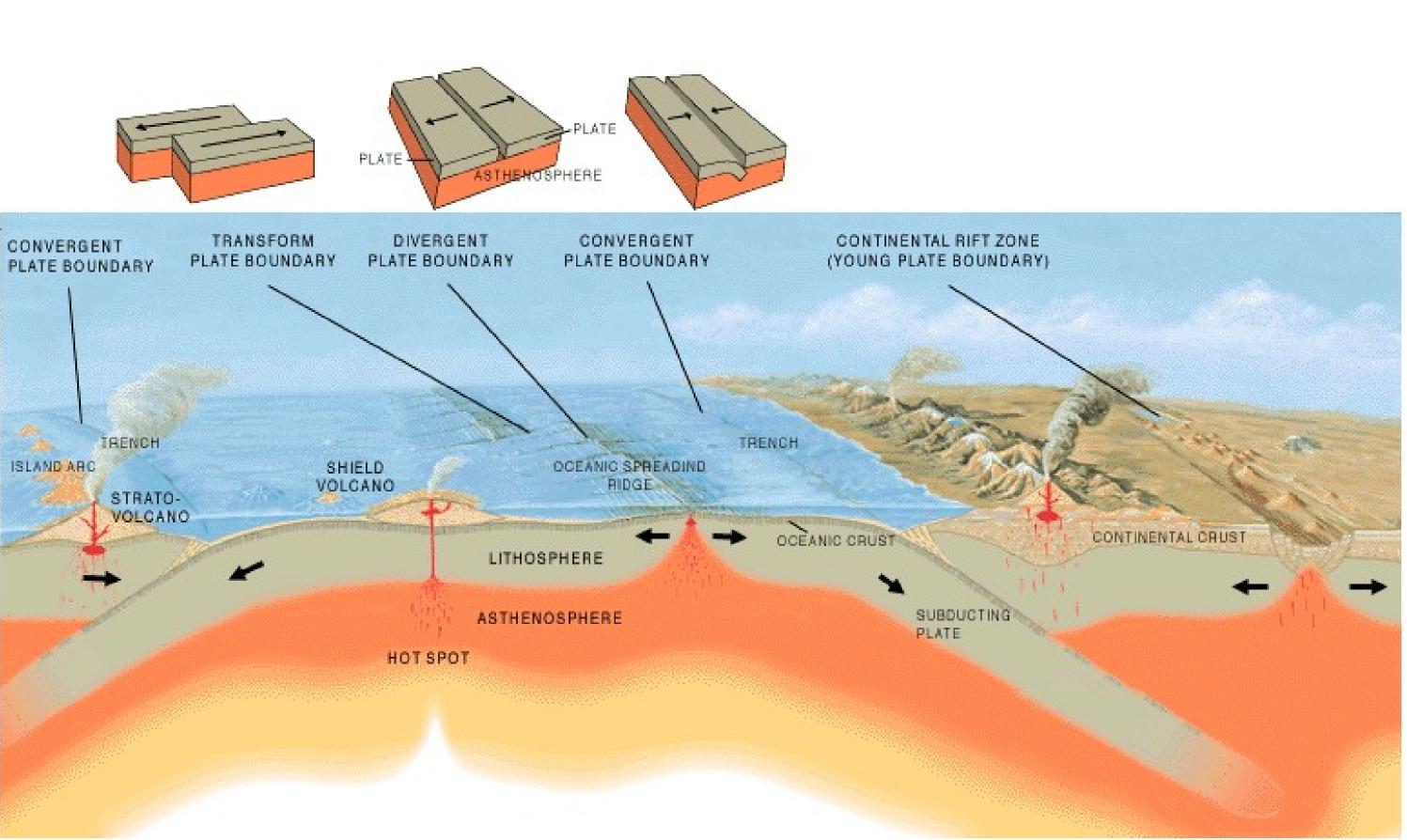




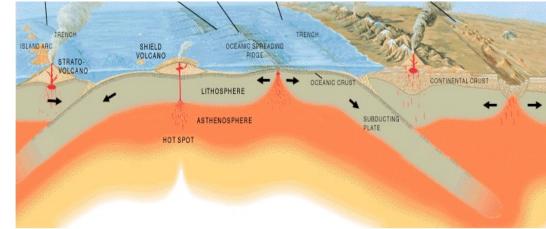


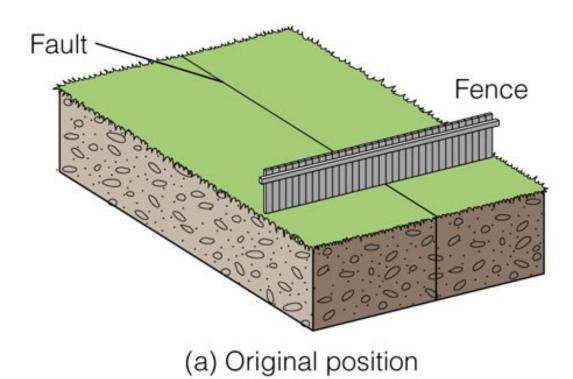
Why earthquakes happen?

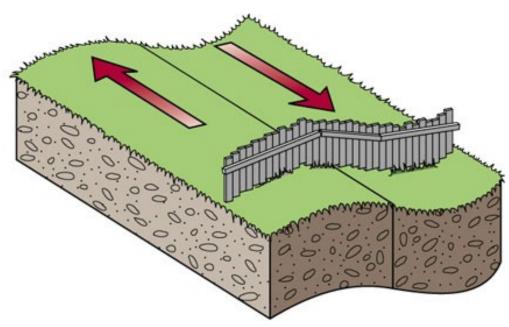




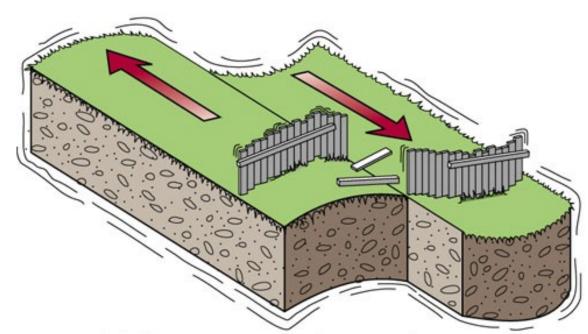








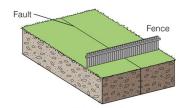
(b) Deformation

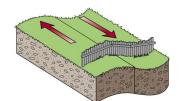


(c) Rupture and release of energy

(d) Rocks rebound to original undeformed shape

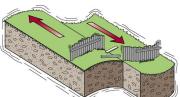


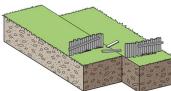


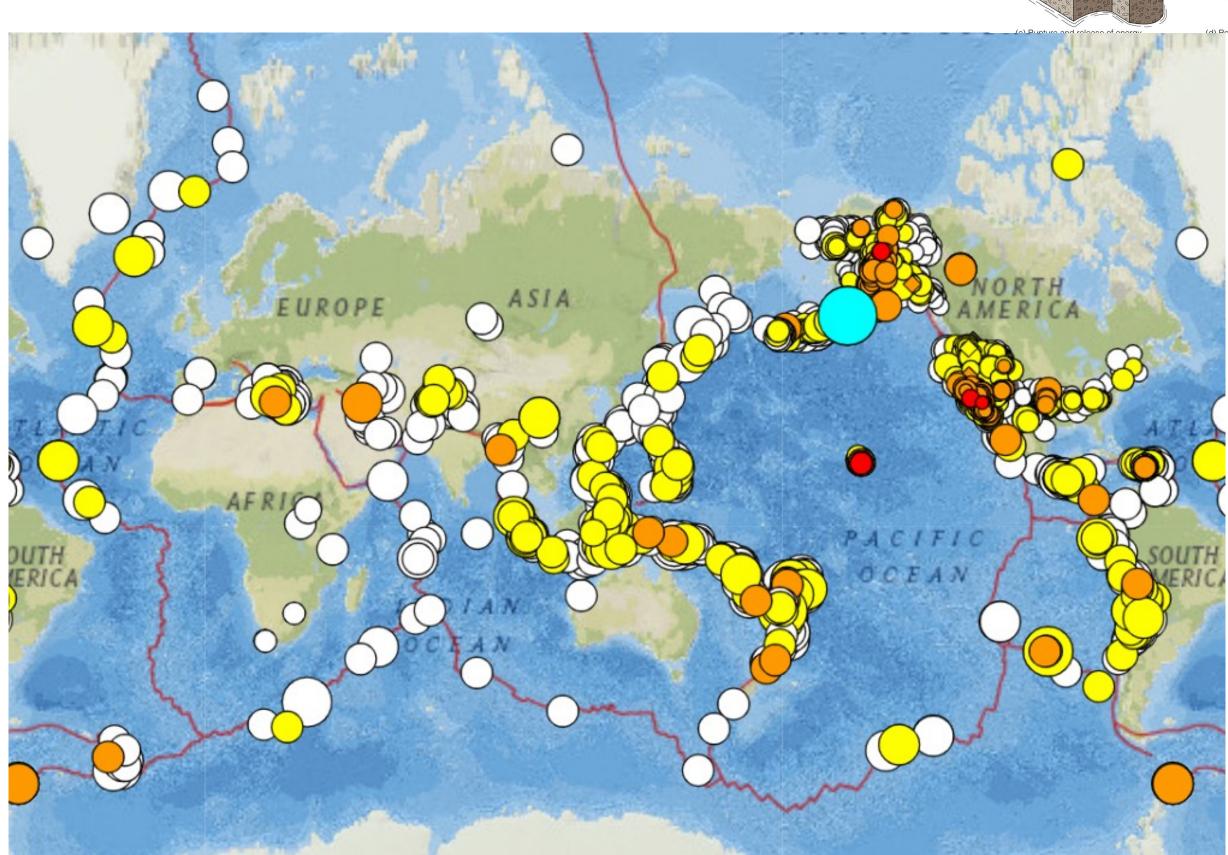


(a) Original position









How do we measure an earthquake?

MAGNITUDE

- · Express in Richter Scale
- · Cause
- · Size of the event

INTENSITY

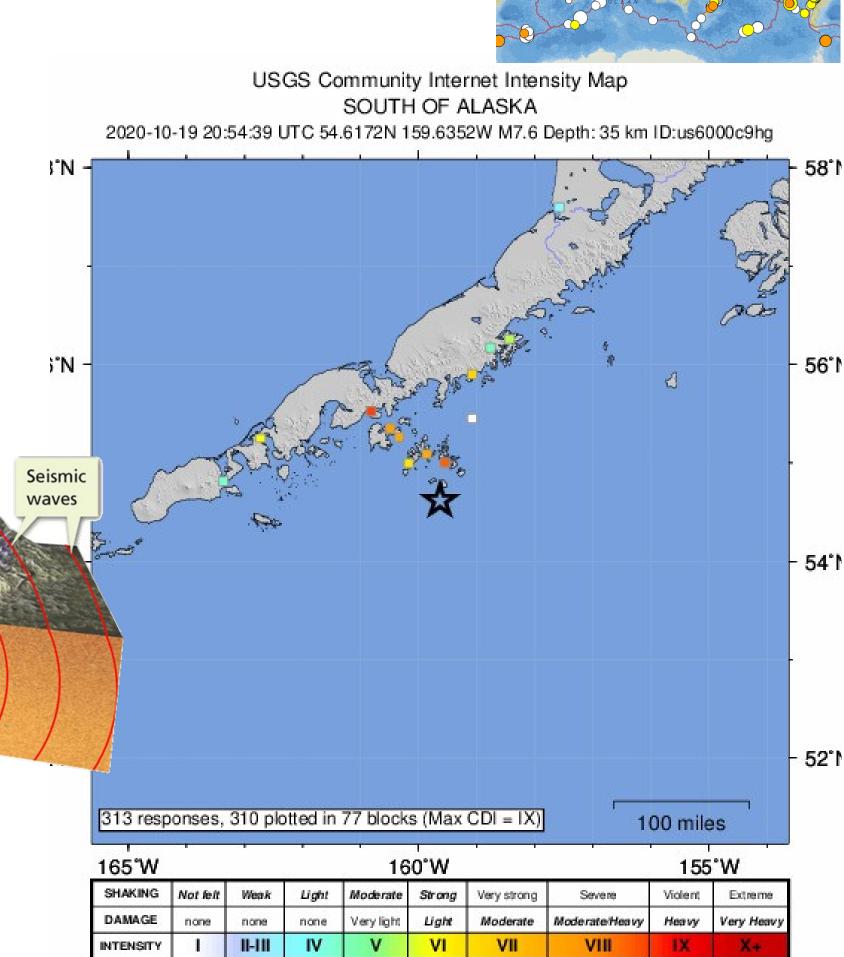
· Express in Modified Mercalli Scale

Denali fault

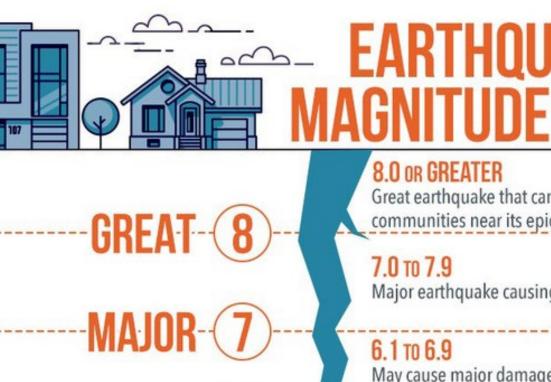
Focus

Epicenter

- · Effect
- Damage potential of the event







STRONG

LIGHT



Great earthquake that can totally destroy communities near its epicenter

Major earthquake causing serious damage

May cause major damage in populated areas

5.5 TO 6.0

Slight damage to buildings

2.5 TO 5.4

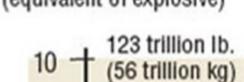
Often felt, but only causes minor damage

2.5 OR LESS

Usually not felt but can be recorded by seismograph

Source: UPSeis / Michigan Tech

Energy release (equivalent of explosive)



4 trillion lb. 9 (1.8 trillion kg)

123 billion lb. 8 (56 billion kg)

4 billion lb. (1.8 billion kg)

123 million lb. 6 (56 million kg)

Magnitude

4 million lb. 5 (1.8 million kg)

12,300 lb. (56,000 kg)

> 4,000 lb. (1,800 kg)

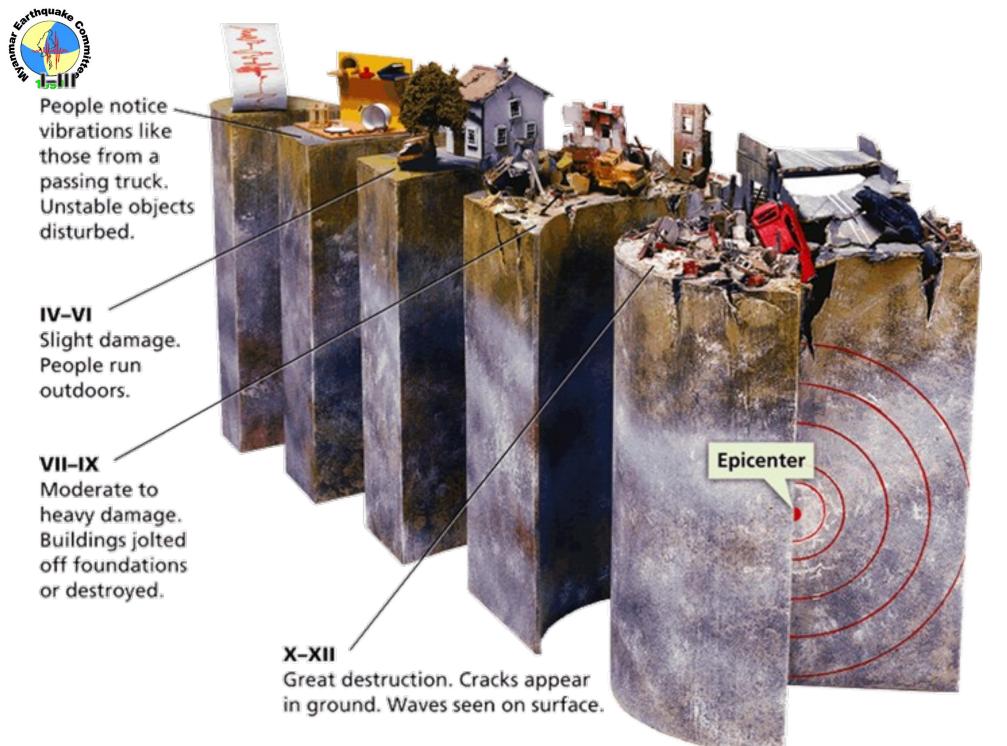
123 lb. (56 kg)

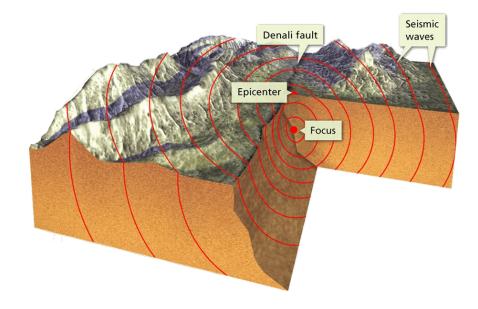
3

MCT

MAGNITUDE

- Express in Richter Scale
- Cause
- Size of the event
- Interest of the Scientists





INTENSITY

- Express in Modified
 - Mercalli Scale
- · Effect
- · Damage potential of the event
- · Interest of the Engineers

| INTENSITY | | II-III | IV | ٧ | VI | VII | VIII | IX | X+ |
|-----------|----------|----------|---------|-------------|--------|-------------|-----------------|---------|------------|
| Shaking | Not felt | Weak | Light | Moderate | Strong | Very Strong | Severe | Violent | Extreme |
| Damage | None | None | None | Very slight | Light | Moderate | Moderate/ heavy | Heavy | Very heavy |
| Peak Acc | <0.17 | 0.17-1.4 | 1.4-3.9 | 3.9-9.2 | 9.2-18 | 18-34 | 34-65 | 65-124 | >124 |
| Peak Vel | <0.1 | 0.1-1.1 | 1.1-3.4 | 3.4-8.1 | 8.1-16 | 16 - 31 | 31-60 | 60-116 | >116 |

Peak Acc = Peak ground acceleration (g), Peak Vel = Peak ground velocity (cm/s)



Earthquakes in our world

esuvius AD62

These two prosperous Roman cities had not yet recovered from the quake of 62 when they were buried by the eruption of Mount Vesuvius in 79. When Vesuvius erupted in AD79 it obliterated two Roman cities, Pompeii and Herculaneum, killing most of the inhabitants and permanently altering the landscape around them.



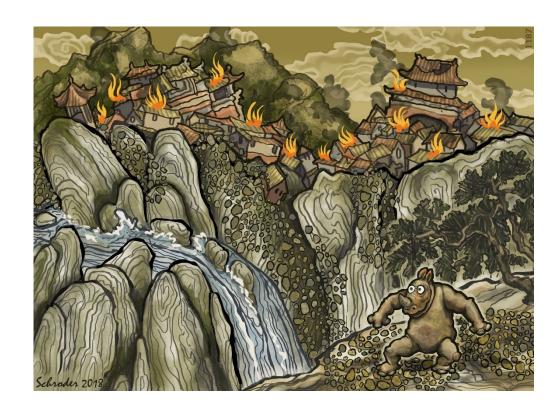


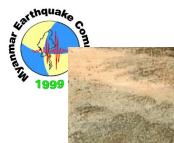
Aleppo Earthquake 1138

Aleppo is situated northern portion of the Dead Sea and it separates the African plate from the Arabian Plate. The series of the earthquakes took place from October 1138 to the month of the June 1139. The area that was the hit the most was Harim. The earthquake caused deaths of around 230,000 people.

Shaanxi earthquake 1556

The Shaanxi earthquake that hit China in 1556, the deadliest earthquake on record, wiped out almost entire counties and killed around 830,000 people. Records describe how the landscape was completely changed, with new mountains and valleys appearing and rivers changing course.







Lisbon Earthquake 1755

The Lisbon earthquake and tsunami was one of the largest earthquakes the modern era has seen, with a possible magnitude of 9 on the moment magnitude scale. This would be equivalent to the Indian Ocean earthquake and tsunami of 2004.

The 1755 earthquake virtually destroyed Lisbon, with as many as 100,000 people killed. Massive fissures up to 5 meters (16 ft) wide opened in the city. Survivors hurried to the port area, which was relatively open and unscathed, only to be met with a 30-meter-high (100 ft) tsunami.

The Age of Enlightenment is directly linked to the events on November 1, 1755, which was the celebration of All Saints' Day, as the disaster destroyed nearly every religious building and church throughout Lisbon and, more importantly, Portugal. [10]

Immanuel Kant, Jean-Jacques Rousseau, and many others took inspiration from the earthquake and led us to our cultural, political, ideological, and industrial revolutions in Europe.

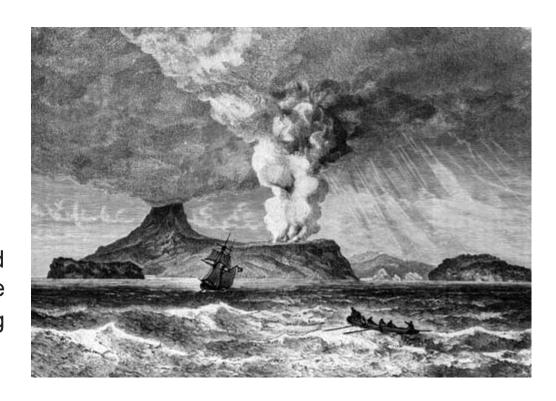
This reasoning became the primary source of authority and legitimacy. It came to advance ideals like liberty, progress, tolerance, fraternity, constitutional government, and the separation of church and state.

Krakatoa 1883

It is an island of Indonesia, located in the Sunda Straight.

A series of eruptions began on August 27th, proving to be some of the most damaging volcanic activity in recorded history.

The death toll is estimated to be around the twelve-hundred thousand mark. Not only did lava flows and <u>ash affect the death toll</u>, but the eruptions also created a series of tsunamis around the island, making things a little more complicated for people trying to flee the scene.





Great Kanto Quake 1923

The Great Kanto Quake which struck Japan in 1923 killed hundreds of thousands of people and devastated Tokyo so much so that the government even considered moving the capital somewhere else.

Ultimately, the disaster sparked both soul-searching and nationalism in Japan. Just eight years later, the nation took its first steps toward World War II with the invasion and occupation of <u>Manchuria</u>.





Tangshan earthquake 1976

The Tangshan earthquake in 1976 was one of the deadliest earthquakes in China's history, causing around 242,000 people to lose their lives. The political repercussions of the disaster, however, went far beyond the death toll and damage.

The quake is seen as a key contributor to the end of the Cultural Revolution in China – one of the most violent eras in Chinese history, and the arrest of the 'Gang of Four', who were blamed for implementing the regime's harsh policies.





Indian Ocean Tsunami 2004

Aceh province, Sumatra, Indonesia 9.1 ... 200,000 The deaths resulting from this offshore quake actually were caused by a tsunami originating in the Indian Ocean that, in addition to killing more than 150,000 in Indonesia, killed people as far away as Sri Lanka and Somalia.

TŌHOKU EARTHQUAKE AND TSUNAMI, FUKUSHIMA DAIICHI NUCLEAR DISASTER (2011)

A magnitude 9.0 earthquake off the coast of Japan triggered a tsunami wave that rose 133 feet at its highest and traveled as far as six miles inland – much larger and more powerful than expected.

That alone would have been cataclysmic enough, but the event also triggered a technological disaster on the scale of the infamous 1986 Chernobyl crisis: a series of nuclear meltdowns and a large-scale release of radioactive material from the Fukushima Daiichi power plant. Although estimates of the death toll vary, as many as 20,000 people were killed.





Nepal 2015

Kathmandu, Nepal 7.8 IX 9,000 The Nepal earthquake of 2015 was accompanied by two aftershocks of magnitude 6.6 and 6.7 within the first hour after the quake. A magnitude-7.3 aftershock struck the region on May 12, killing more than 100 people.



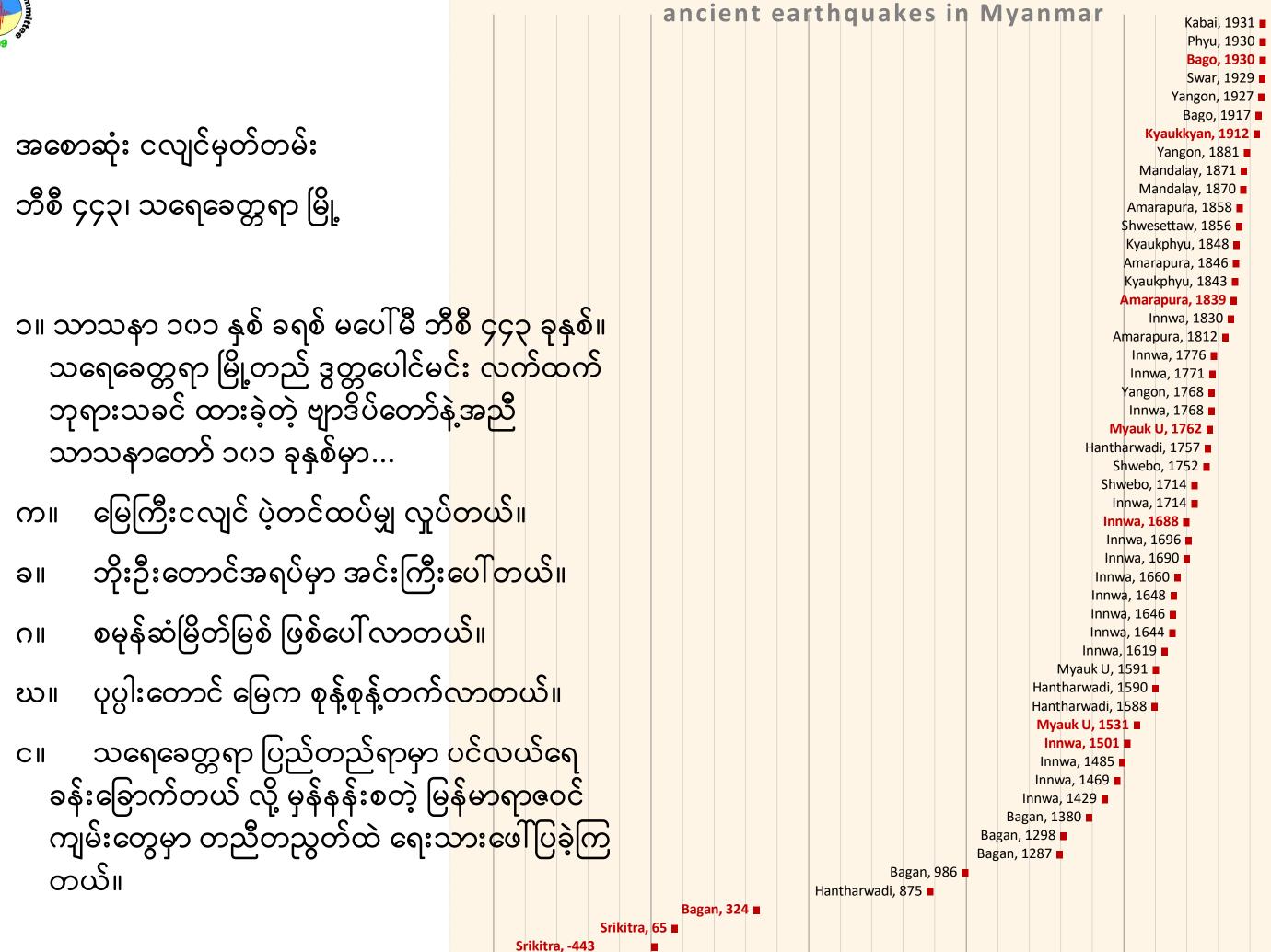
Earthquakes in Myanmar

ကိုးကားချက်။ သမိုင်းဝင် ငလျင်ကြီးများနဲ့ ပုဂံ ရွှေကိုင်းသား

(စနေစာပေဝိုင်း၊ ဟံသာဝတီသတင်းစာ ပုံနှိပ်တိုက်၊ မေလ ၁ ရက် ၁၉၇၆၊ စာ ၂၆၇-၂၉၃)







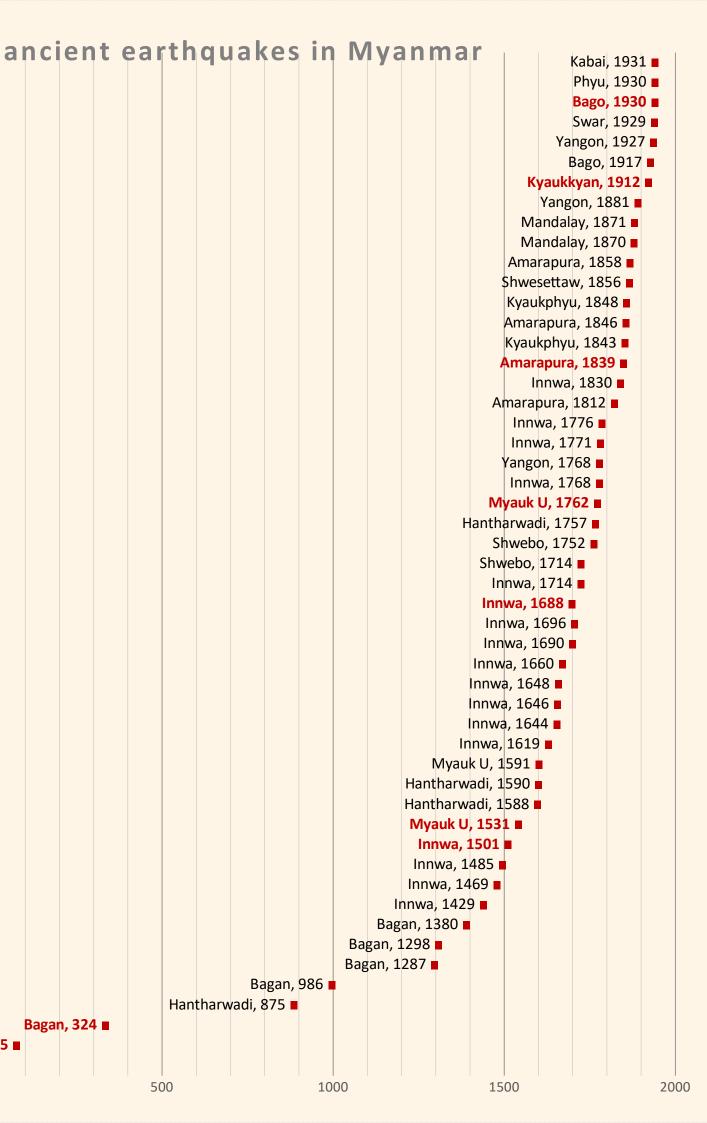


၁၇၆၂၊ ရခိုင်ငလျင်

"ခရစ် ၁၇၆၂ ခုနှစ် ဧပြီလအတွင်း နောင်တော်ကြီးမင်း လက်ထက်တွင် မြေငလျင်လှုပ်ရာ မြန်မာနိုင်ငံ ကိုသာ မက ဘင်္ဂလားပင်လယ်အော်၏ အရှေ့ဘက်ကမ်း တလျှောက်လုံးပါ လှုက်သည်ဟု အမှတ်အသား ရှိ သည်။ ထိုငလျင်ကြောင့် ရခိုင်တိုင်း ကမ်းရိုးတန်း အချို့နေရာဒေသတို့သည် မူလ မြေမျက်နှာပြင် ထက် ပို၍ တက်ကြွ လာခဲ့သည်။

Srikitra, 65 ■

Srikitra, -443



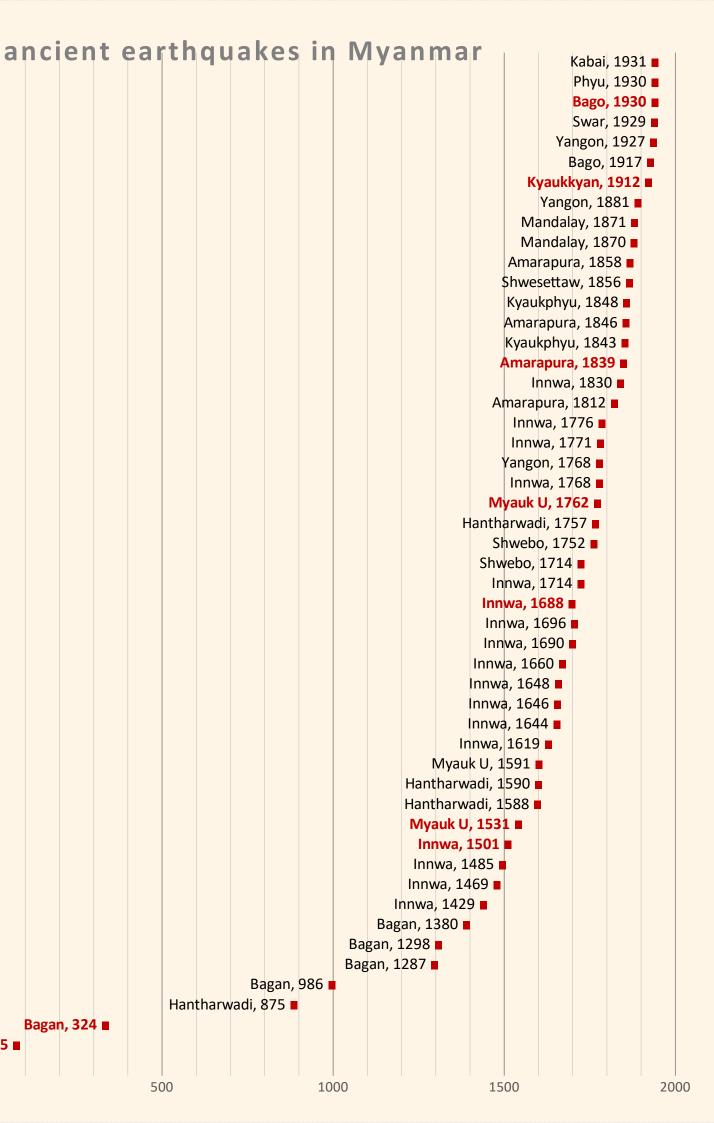


၁၈၃၉၊ အင်းဝငလျင်

"သာယာဝတီမင်လက်ထက် ၁၈၃၉ ခုနှစ် မတ်လ အတွင်း မြေငလျင်လှုပ်ရာ မင်းကွန်းစေတီတော်ကြီး နှင့်အတူ မင်းကွန်း ခေါင်းလောင်းတော်ကြီး ပြိုကွဲပျက် စီးခဲ့ရသည်မှာ ယနေ့တိုင်ပင် ကျွန်ုပ်တို့ မျက်မြင်ဒိဋ္ဌ ဖြစ်သည်။ ထိုစဉ်က အင်းဝရွှေနန်းတော်ဟောင်းနှင့် အမရပူရ ရွှေနန်းတော် တို့သည်လည်း ပျက်စီးသည်။ ထိုငလျင်မှာ ဗန်းမော် ဒေသမှသည် ရန်ကုန်သို့ တိုင်အောင် လေးငါးရက် ဆက်တိုက်လှုပ်သော ငလျင် ဖြစ်သည်။

Srikitra, 65 ■

Srikitra, -443



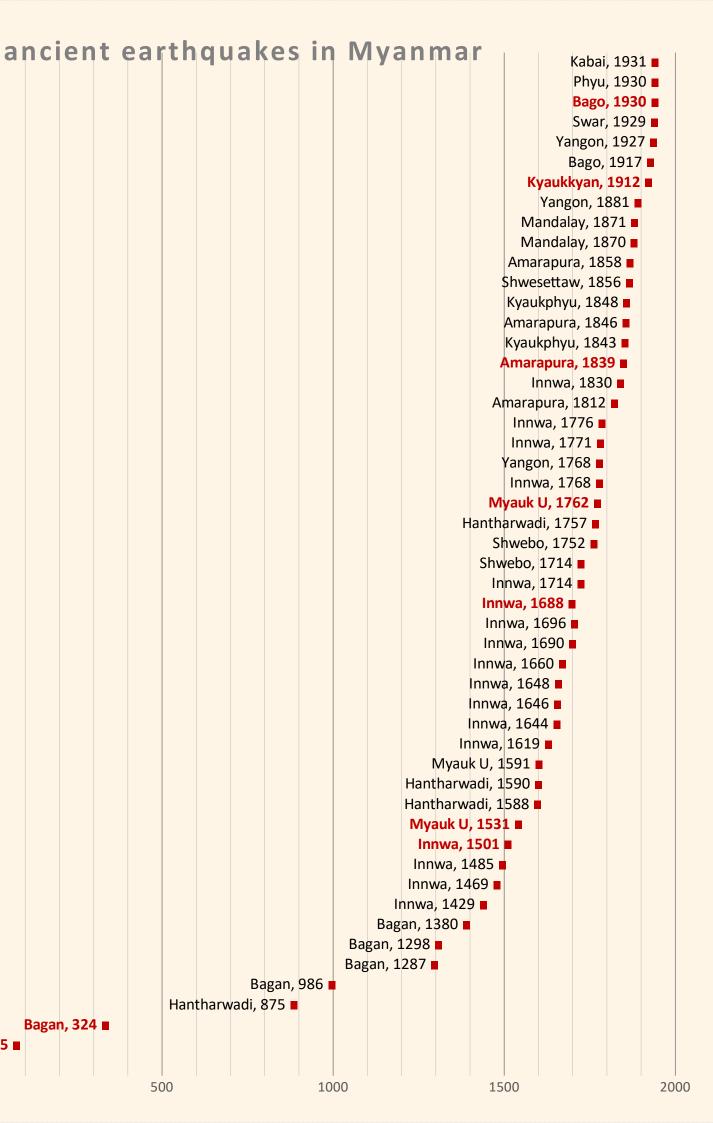


၁၉၁၂၊ မေမြို့ငလျင်

"ထို့နောက် ဗြိတိသျှခေတ် ၁၉၁၂ ခု မေလ ၂၃ ရက်တွင် ရှမ်းပြည်နယ်ရှိ ကျောက်ကြမ်းဒေသတဝိုက်ကို ဗဟို ပြု၍ မြေငလျင်လှုပ်ရာ မန္တလေး၊ မိုးကုတ်၊ ရွှေဘို၊ စစ် ကိုင်း၊ အောက်ချင်းတွင်း၊ ကျောက်ဆည်၊ မြင်းခြံ၊ မိထ္ထီ လာ၊ မကွေး၊ ရမည်းသင်း၊ တောင်ငူ၊ ပဲခူး စသည်တို့ အပြင်းအထန် ခံရ၍ ၎င်းမြေငထျင်သည် ယိုးဒယား နိုင်ငံ သို့ပင်လျှင် ပြန့်နှံသွားသည်။

Srikitra, 65 ■

Srikitra, -443





ancient earthquakes in Myanmar Kabai, 1931 ■ Phyu, 1930 ■ Bago, 1930 ■ Swar, 1929 ■ Yangon, 1927 ■ Bago, 1917 ■ Kyaukkyan, 1912 ■ Yangon, 1881 ■ Mandalay, 1871 ■ Mandalay, 1870 ■ Amarapura, 1858 ၁၉၂၉-၃၀၊ ပဲခူးငလျင် \$hwesettaw, 1856 ■ Kyaukphyu, 1848 Amarapura, 1846 🛮 Kyaukphyu, 1843 ■ Amarapura, 1839 ■ Innwa, 1830 **■** Amarapura, 1812 ''၁၉၂၉ ခုနှစ်ဩဂုတ်လ ၈ရက်တွင် တေ<mark>ာင်ငူအပိုင် ဆွာမြို့တွင် ငလျင်လ</mark>ူပ် Innwa, 1776 **■** Innwa, 1771 **■** ရာ လူတို့အတော်ပင် အထိတ်တလန့် ဖ<mark>ြစ်ခဲ့ရသည်။</mark> Yangon, 1768 ■ Innwa, 1768 **■** Myauk U, 1762 ■ ''သို့သော် မြန်မာနိုင်ငံတွင် အပြင်းထန်<mark>ဆုံး၊ အပျက်စီးဆုံး၊</mark> Hantharwadi, 1757 Shwebo, 1752 ■ အသေအပျောက် အများဆုံး ငလျင်မှာ <mark>၁၉၃၀ပြည့်နှစ် မေလ ၅ ရက်တွင</mark>် Shwebo, 1714 ■ Innwa, 1714 ■ Innwa, 1688 **■** လှုပ်သော ပဲခူးမြေငလျင်ပင် ဖြစ်သည်။ နွေမြော်ဧရာ စေတီကြီး ပြုကျ Innwa, 1696 Innwa, 1690 ပျက်စီးသည်။ ပဲခူး၌ လူပေါင်း ၅၀၀ ကျော်ခန့် သေကြေပျက်စီး၍ ရန်ကုန် Innwa, 1660 ■ Innwa, 1648 ■ မြို၌ပင်လျှင် အချို့တိုက်များပြု၍ လူအ<mark>နည်းငယ် သေကြေ ပျက်စီးသည်။</mark> Innwa, 1646 Innwa, 1644 Innwa, 1619 ■ ထိုငလျင်များမှာ မြန်မာနိုင်ငံ အောက်ပို<mark>င်းဒေသကို ပို၍ အထိအခိုက် မျာ</mark>း Myauk U, 1591 Hantharwadi, 1590 စေခဲ့သည်။ အထူသဖြင့် ပဲခူးနယ်တဝိုက<mark>် ကျောက်ဖြူ၊ မြိတ် စသည့်နေရ</mark>ာ Hantharwadi, 1588 Myauk U, 1531 ■ တို့တွင် ပြင်းထန်၍ မြောက်ပိုင်း ရှမ်းပြ<mark>ည် မိုးမိတ်နှင့် တောင်ပိုင်း ရှမ်းပြည</mark>် Innwa, 1501 ■ Innwa, 1485 Innwa, 1469 ■ တို့တွင်လည်း အနည်းငယ် လှုပ်ခဲ့သည်။ Innwa, 1429 ■ Bagan, 1380 **■** Bagan, 1298 ''ထို့နောက် ခြောက်လကျော်ခန့် ကြာသေ<mark>ာအခါ ဖြူးမြို့တွင် ၁၉၃၀ ပြည့</mark>် Bagan, 1287 ■ နှစ် ဒီဇင်ဘာလ ၃ ရက်၌ မြေငလျင် ကြီး<mark>စွာလှုပ်ရာ လူပေါင်း ဥပ္ပ နေ့ကျ</mark>ာ် Hantharwadi, 875 ■ သေကြေပျက်စီးခဲ့သည်။ Srikitra, -443 2000



ancient earthquakes in Myanmar Kabai, 1931 ■ Phyu, 1930 Bago, 1930 ■ Swar, 1929 ■ Yangon, 1927 ■ Bago, 1917 ■ Kyaukkyan, 1912 ■ Yangon, 1881 Mandalay, 1871 ■ Mandalay, 1870 ■ Amarapura, 1858 ၁၉၇၅၊ ပုဂံငလျင် \$hwesettaw, 1856 ■ Kyaukphyu, 1848 Amarapura, 1846 ■ Kyaukphyu, 1843 ■ Amarapura, 1839 ■ Innwa, 1830 **■** Amarapura, 1812 ပုဂံမြို့က ကိုယ်တွေ့တဦးရဲ့ ပြောပြချက်<mark>က ပထမမြေကြီးကြွ</mark> Innwa, 1776 Innwa, 1771 **■** တက်လာပြီး ဘယ်ညာ သုံးကြိမ်ခန့် ခပ<mark>်ပြင်းပြင်း လှုပ်တယ်။</mark> Yangon, 1768 ■ Innwa, 1768 **■** နောက် တော်လဲသံကြီး ကြားရတယ်။နေ<mark>ာက်ဘုရားတွေ တအုံ</mark>း Myauk U, 1762 ■ Hantharwadi, 1757 အုံးနဲ့ ပြိုကျပီး အုက်မှုန့် သဲမှုန့် မြေမှုန့် <mark>တွေ လေထဲလွင့်ပျ</mark>ံ Shwebo, 1752 ■ Shwebo, 1714 ■ တက်ကုန်တယ်။ ဒါကိုတချို့က သဲမုန်းကို သူလာတယ်လို့ Innwa, 1714 Innwa, 1688 **■** Innwa, 1696 ထင်ကြတယ်။ Innwa, 1690 Innwa, 1660 ■ Innwa, 1648 ■ သတင်းတပုဒ်ကတော့ ညနေခြောက်န<mark>ာရီကျော်ကျော်မှ</mark>ာ Innwa, 1646 Innwa, 1644 အနောက်တောင်ဒေါင့်က တော်လဲသံကြီး မြည်ဟီးလာပြီး ဗုံး Innwa, 1619 ■ Myauk U, 1591 Hantharwadi, 1590 ပေါက်ကွဲသံလို ကြားရတယ်။ နောက် င<mark>လျင်လှုပ်တယ် လ</mark>ို့ဆို Hantharwadi, 1588 Myauk U, 1531 ■ တယ်။ ပဲခူးတိုင်း မင်းလှမြို့နယ် မှာလဲ <mark>လျှင်မလှုပ်မ</mark>ီ Innwa, 1501 **■** Innwa, 1485 အနောက်တောင်ထောင့်ဆီက မိုးချုန်းသ<mark>ံလိုအသံကြီး မြည်ဟီ</mark>း Innwa, 1469 Innwa, 1429 Bagan, 1380 **■** လာပီးမှ ငလျင်လှုပ်တယ် လို့ဆိုတယ်။ Bagan, 1298 Bagan, 1287 ■ Bagan, 986 Hantharwadi, 875 ■ Bagan, 324 **■** Srikitra, 65 ■ Srikitra, -443

1000

1500

2000



Earthquakes in Myanmar

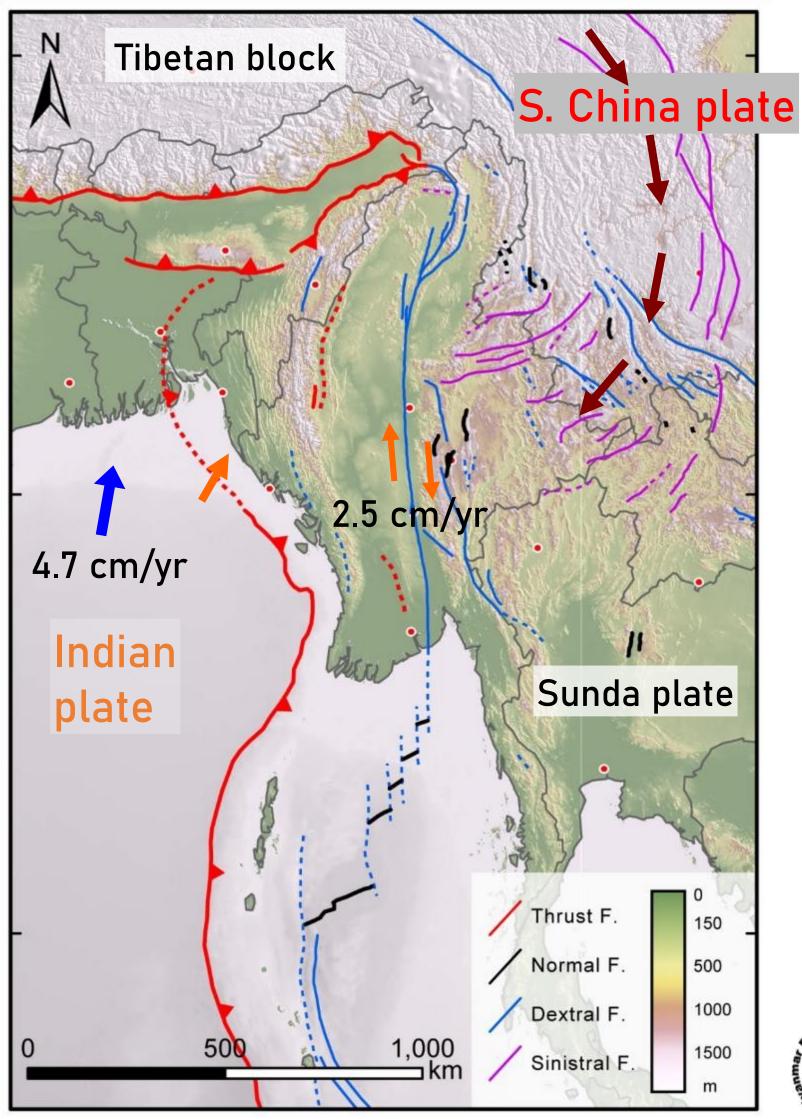
Earthquakes sources in Myanmar

- · Sagaing Fault
- · Subduction Zone
- Shear Zone in Shan Platau



Tectonics of Myanmar

Wang Yu et al., 2013





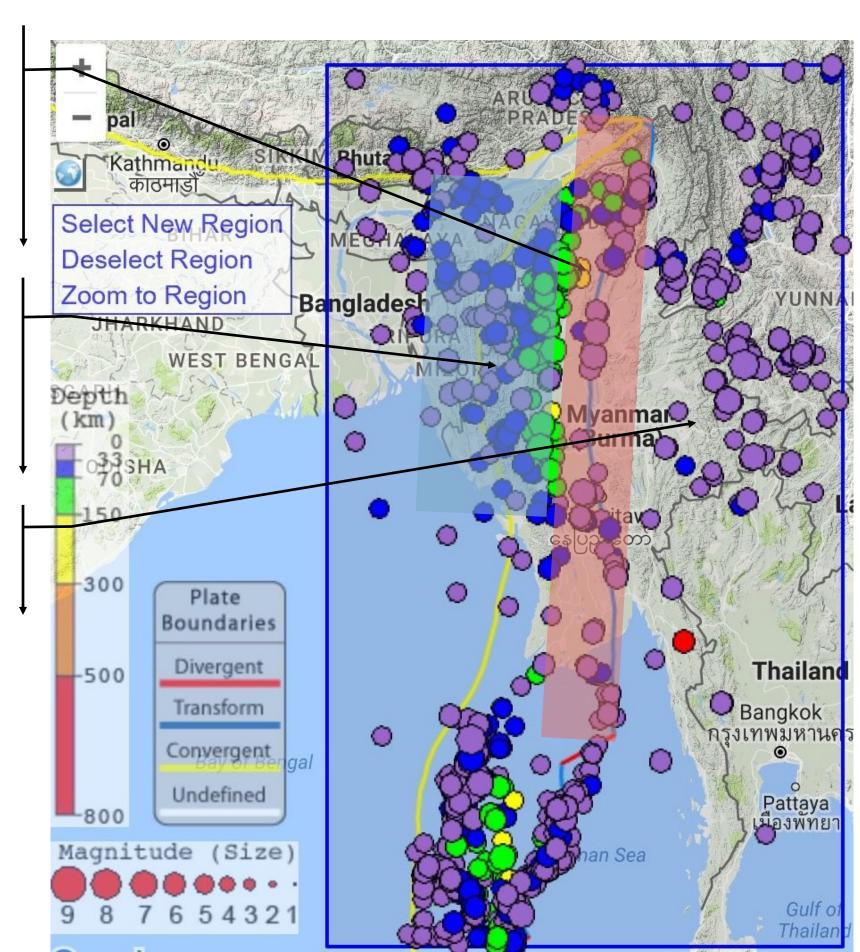


Seismicity of Myanmar (Mag 5 and above)

Sagaing Fault
Strike Slip
Right Lateral
Shallow Earthquakes
2.5 cm/yr

Subduction Zone Trust Reverse Fault Deep Earthquakes 4.7 cm/yr

Secondary Faults
Shallow Earthquakes





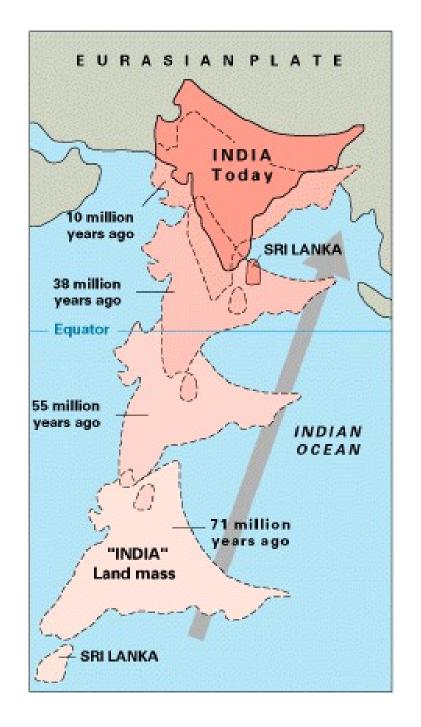
Earthquake Engineering Concept

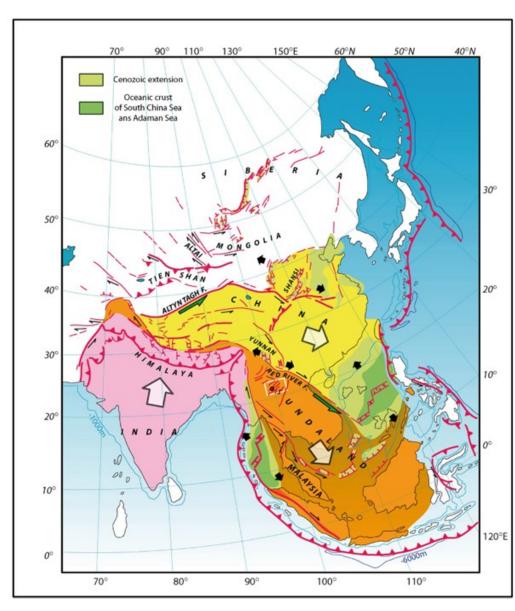
Earthquake Engineering

- Earthquake Hazard
- Capacity of a structure
- Demand vs. Capacity



Earthquake Hazard- Demand Side

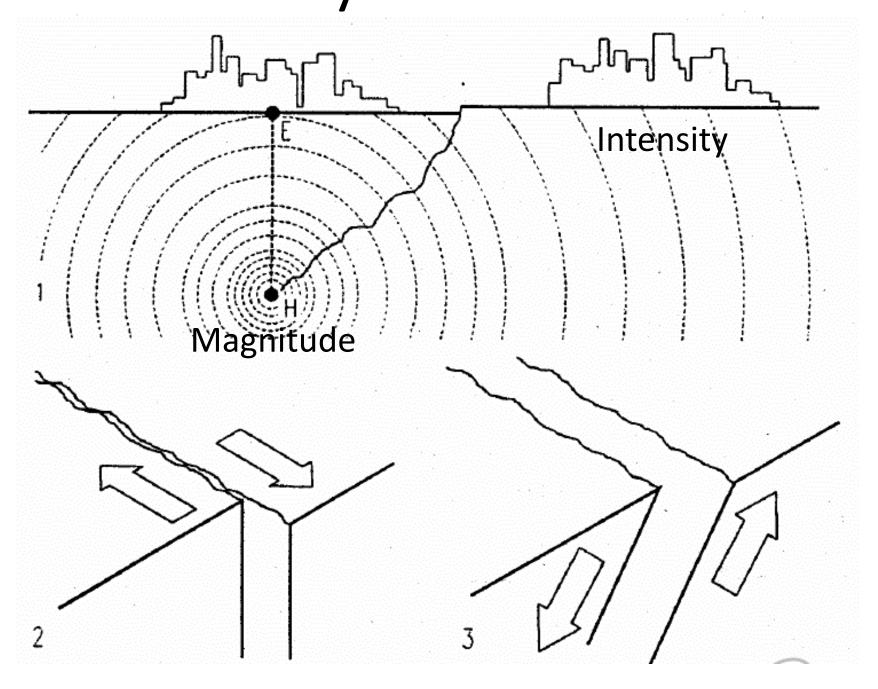




- Earth mass is moving and striking one another.
- Create earthquake



Ground Shaking, Magnitude, Intensity

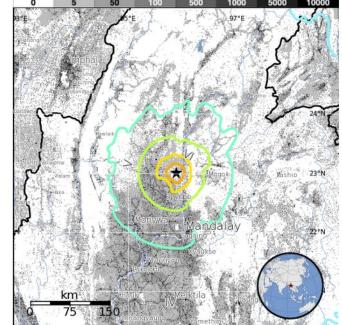


 Sudden earth shaking applies at the base of the buildings



Population Exposure

Population per ~1 sq. km. from LandSca



Structure Information Summary

Overall, the population in this region resides in structures that are highly vulnerable to earthquake shaking, though some resistant structures exist.

Secondary Effects

Recent earthquakes in this area have caused secondary hazards such as landslides that might have contributed to losses.

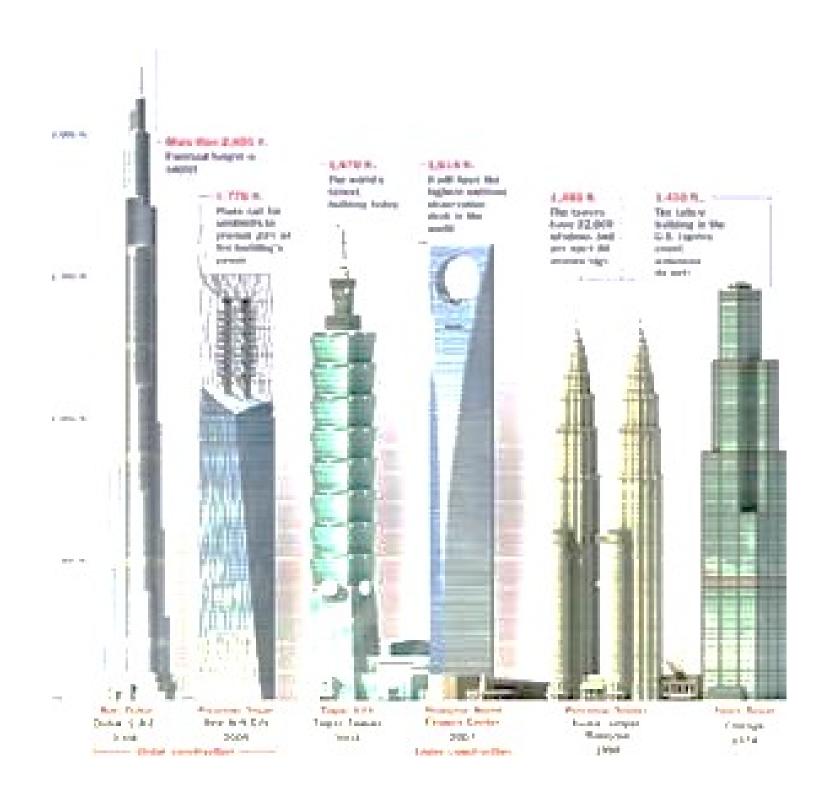
Selected Cities Exposed

from GeoNames Database of Cities with 1,000 or more

| IMM | City | Population | | | | |
|-----|-----------|------------|--|--|--|--|
| VI | Shwebo | 88k | | | | |
| V | Mogok | 90k | | | | |
| V | Mandalay | 1,208k | | | | |
| V | Maymyo | 117k | | | | |
| V | Monywa | 182k | | | | |
| V | Sagaing | 78k | | | | |
| IV | Imphal | 223k | | | | |
| IV | Myitkyina | 90k | | | | |
| IV | Haka | 0 | | | | |
| IV | Taunggyi | 160k | | | | |
| IV | Kohima | 92k | | | | |



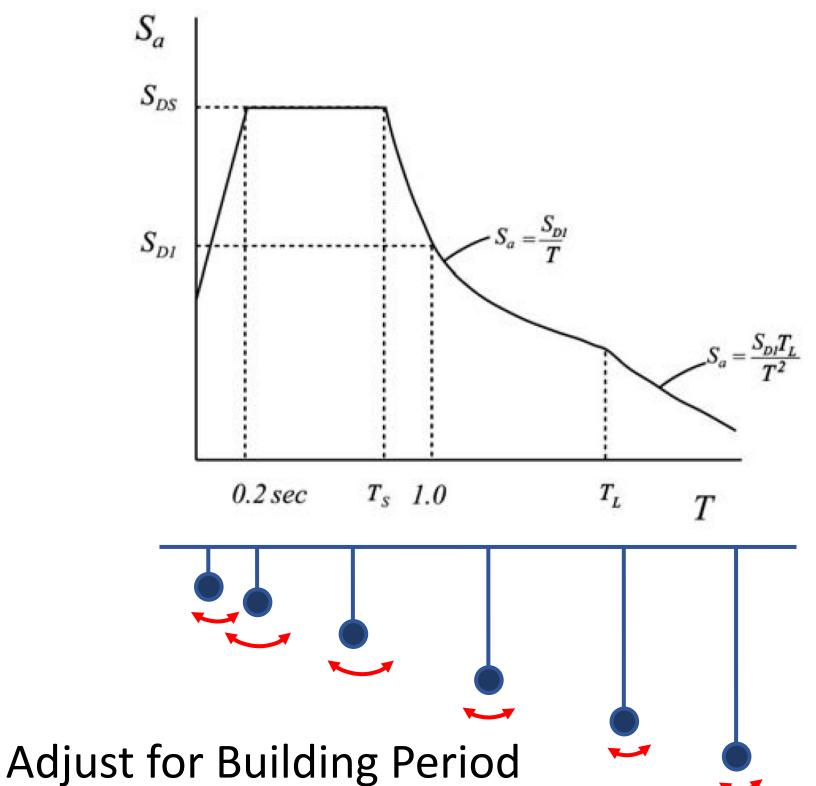
Seismic Response

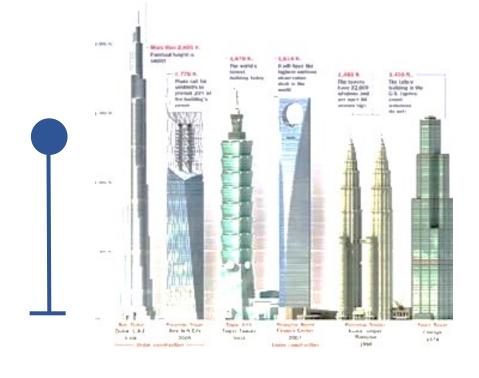






Response Spectrum



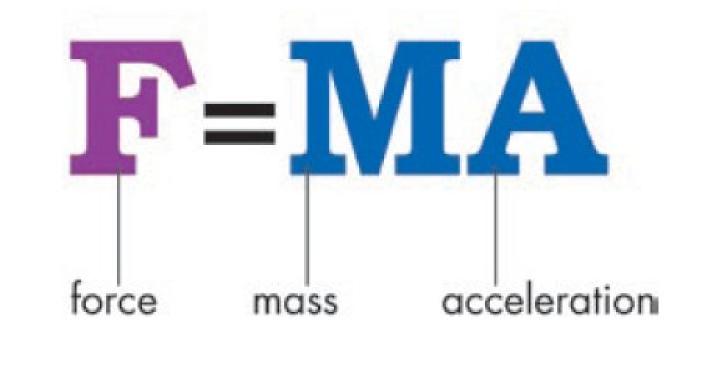


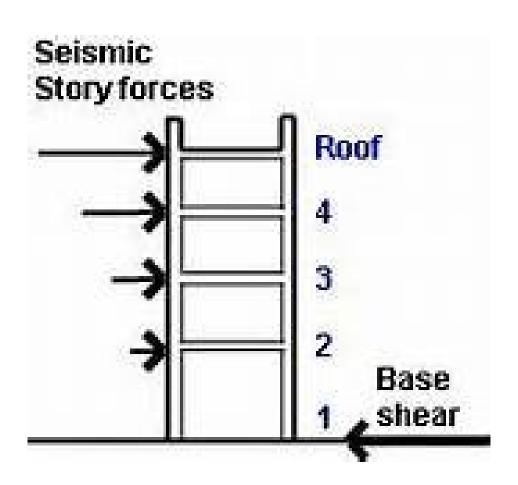


Approximately T = Number of Story / 10



Seismic Forces





- Apply to each level with respect of its story mass.
- Total story forces = Base Shear



Building Response- Capacity Side

1. IN MINOR EARTHQUAKES

WITHOUT SERIOUS DAMAGE

DBE

(Design Basis Earthquake)

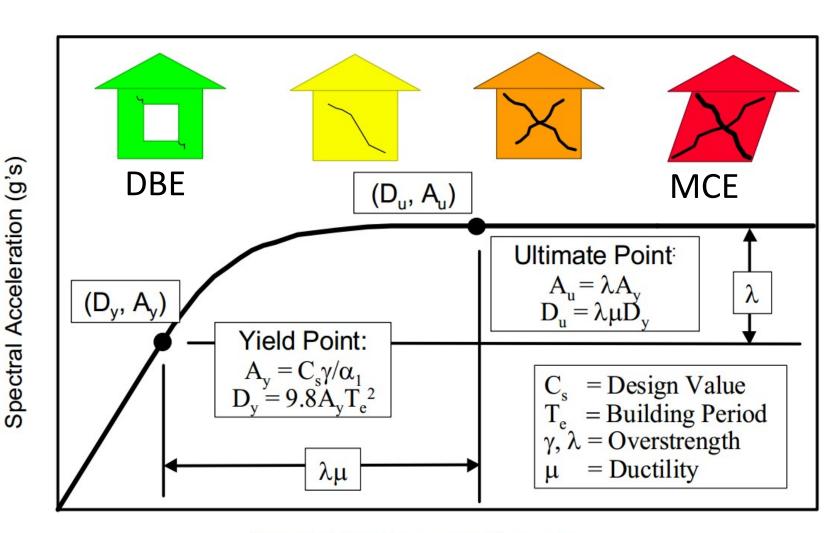
5% in 50 years

2. IN MAJOR EARTHQUAKES WITHOUT STRUCTURAL DAMAGE BUT POSSIBLY EXPERIENCE SOME NON-STRUCTURAL DAMAGE

MCE

(Maximum Credible Earthquake)

2% IN 50 YEARS

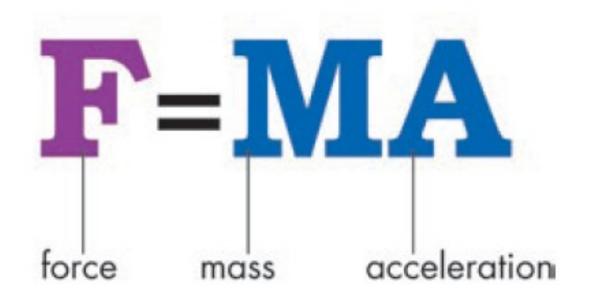


Spectral Displacement (inches)



Demand and Capacity

- Safety Equation
 - Capacity > Demand
 - Capacity = Safety Factor x Demand



Capacity > Demand
Capacity > Demand



Reducing Demand by Mass

F = **m**.a

- Reducing Mass
 - Use lighter materials

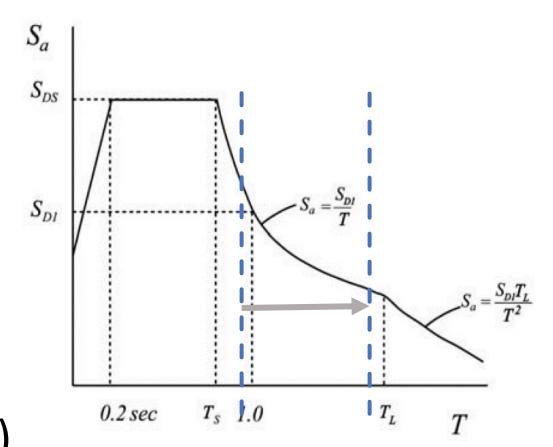




Reducing Demand by Acceleration

F = m.a

- Reducing Acceleration
 - Make more flexible and ductile
 - Use damper (spectrum scale down)
 - Use base Isolation (frequency shift)



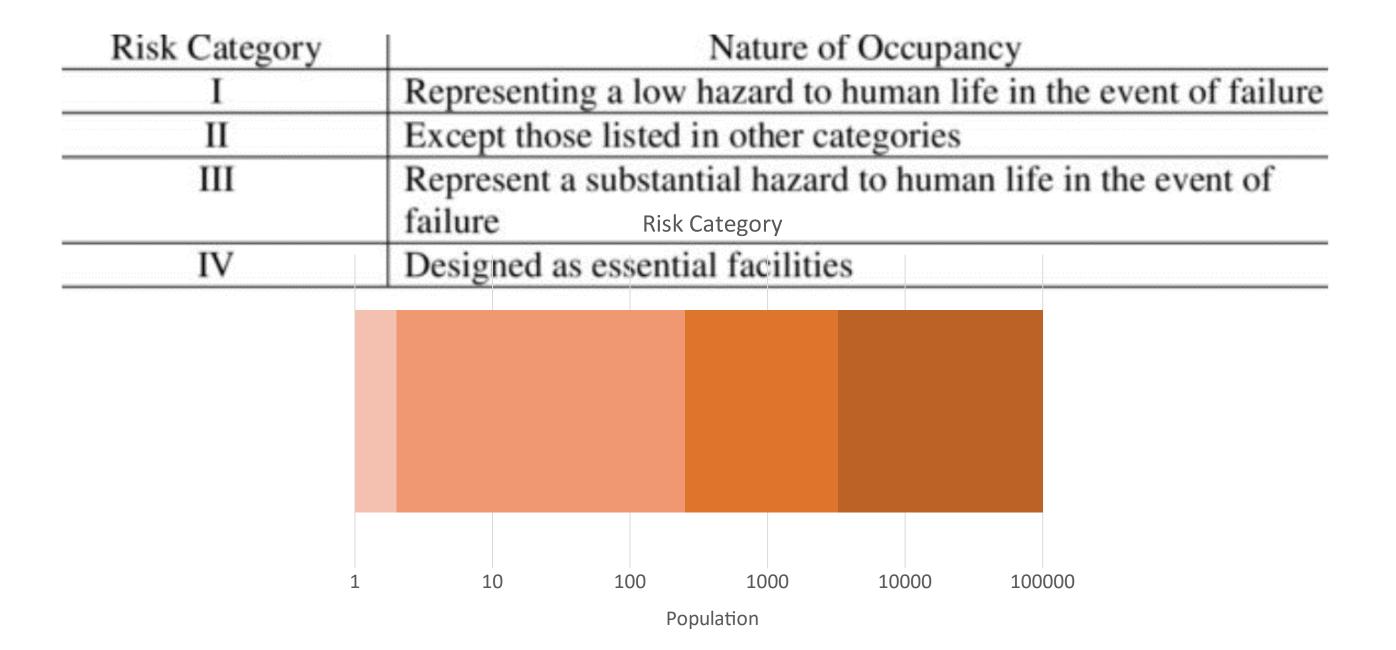
Earthquake Design Concept in MNBC

Myanmar National Building Code

- MNBC Structural Design- IBC-2006 and ASCE7-05 codes.
 - Occupancy Category
 - Load Combinations
 - Earthquake Design Flow Chart

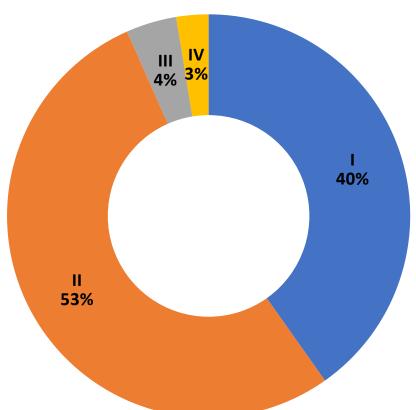


Occupancy Category

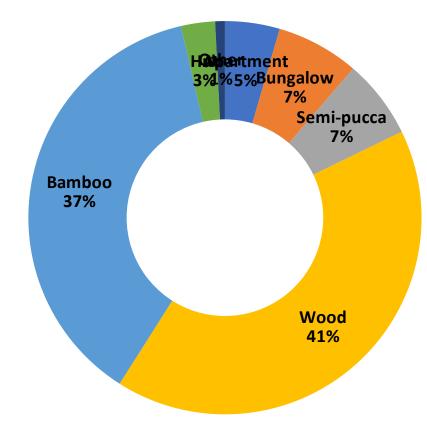




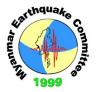




Building Type (Union)



| Cate | | Apartm ent | Bungal ow | Semi- pucca | Wood | Bamboo | Hut | Other | All Buildings |
|------|-----------|---------------|--------------|----------------|--------|--------|-------|-------|------------------|
| I | Temporary | | | | | 37.40% | 2.80% | | 40.20% |
| П | Medium | | 6.12% | 5.85% | 41.20% | | | | 53.17% |
| Ш | High | 4.05% | | | | | | | 4.05% |
| IV | Ess/Haz | 0.45% | 0.68% | 0.65% | | | | 0.80% | 2.58% |
| | Union | 4.50% | 6.80% | 6.50% | 41.20% | 37.40% | 2.80% | 0.80% | 100.00% |



Combinations for Factored Loads

3.2.1.2.2 Basic load combinations

| 1. $1.4 (D + F)$ | Eq. | (3.2.1) |
|------------------|-----|---------|
|------------------|-----|---------|

2.
$$1.2(D+F+T) + 1.6(L+H) + 0.5(L_r \text{ or } R)$$
 Eq. (3.2.2)

3.
$$1.2D + 1.6(L_r \text{ or } R) + (L \text{ or } 0.8W)$$
 Eq. (3.2.3)

4.
$$1.2D + 1.6W + L + 0.5(L_r \text{ or } R)$$
 Eq. (3.2.4)

5.
$$1.2D + 1.0E + L$$
 Eq. (3.2.5)

6.
$$0.9D + 1.6W + 1.6H$$
 Eq. (3.2.6)

7.
$$0.9D + 1.0E + 1.6H$$
 Eq. (3.2.7)



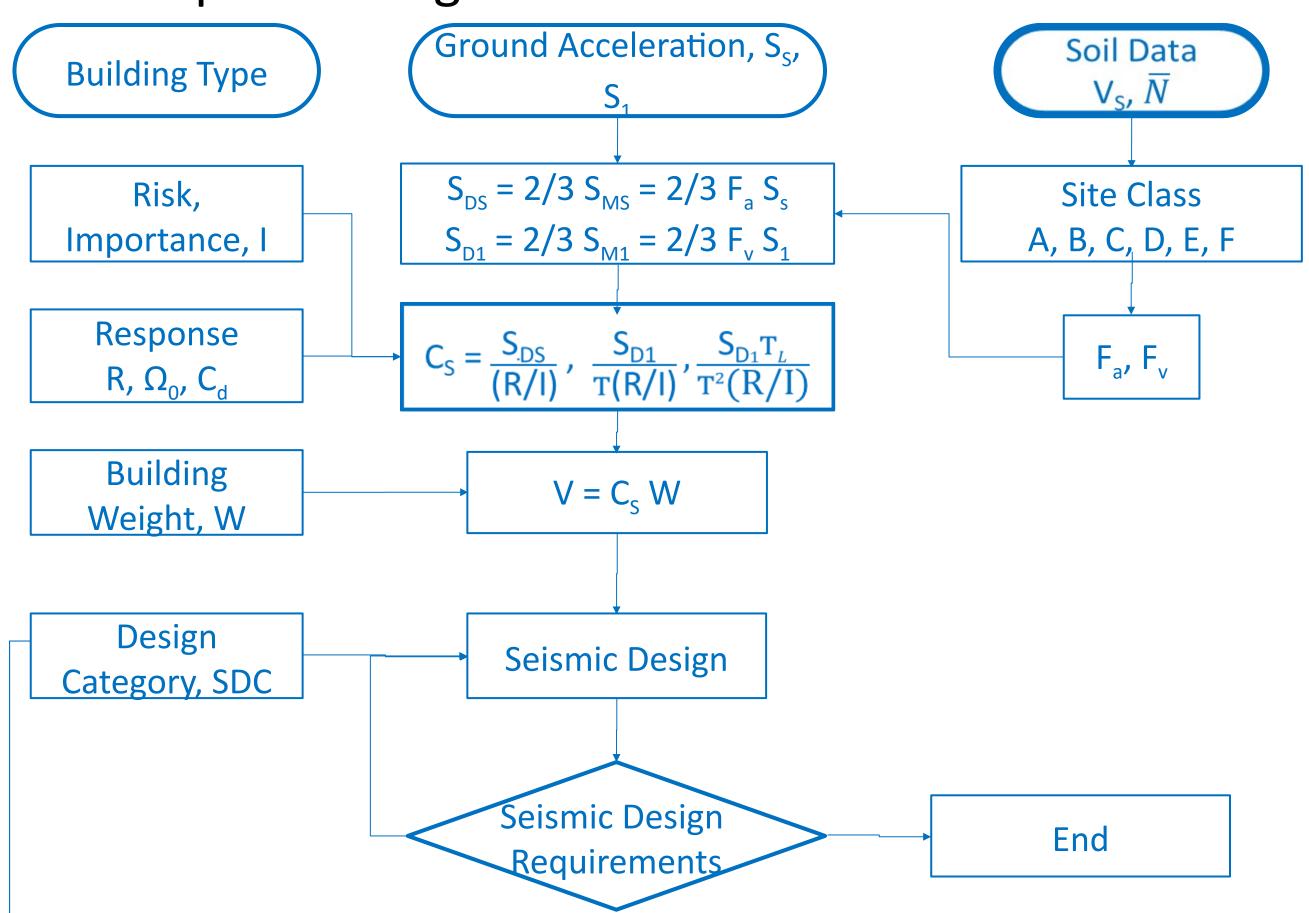
Combinations for Unfactored Loads

3.2.1.3.1 Basic load combinations

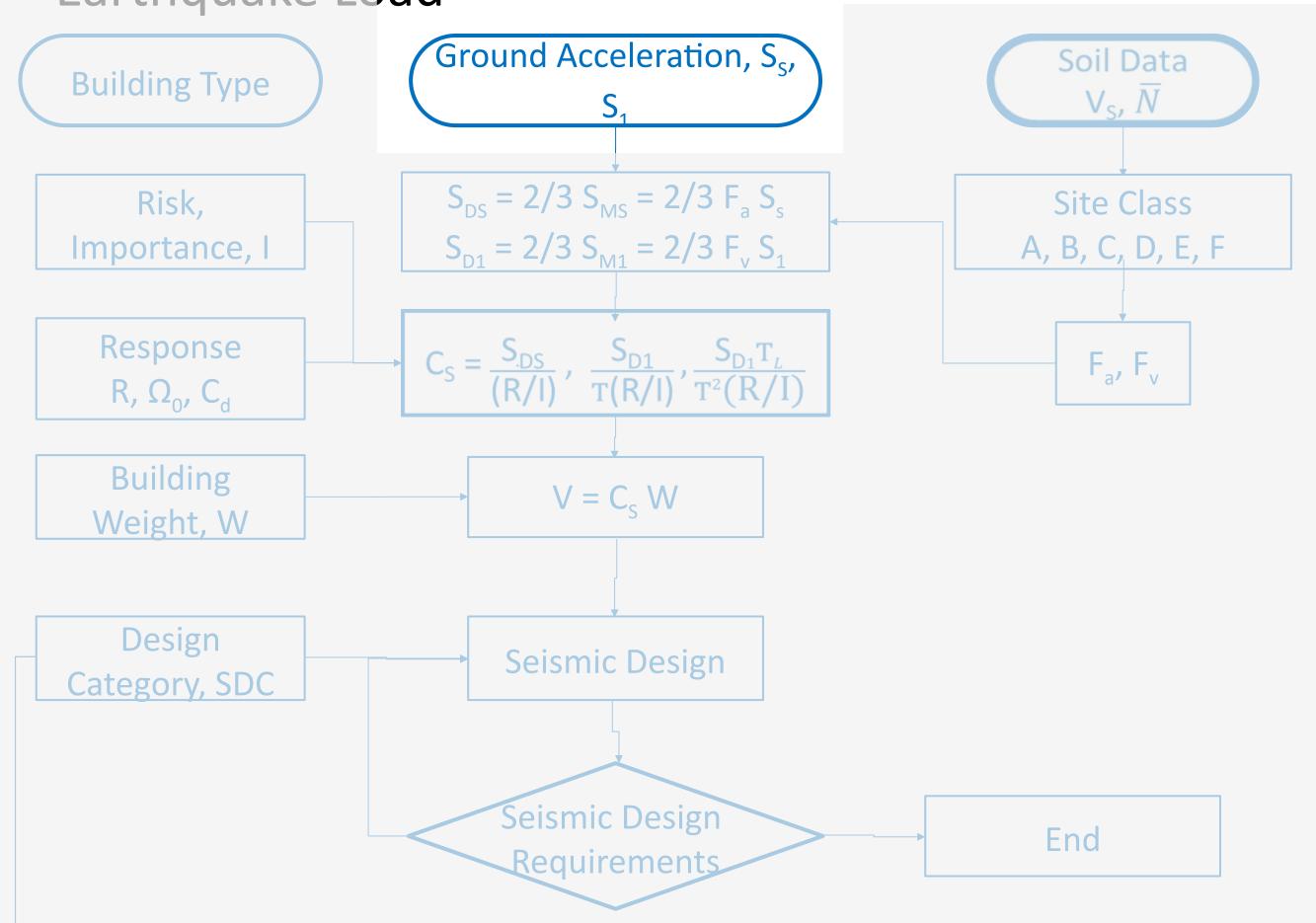
| 1. | D + F | Eq. (3.2.8) |
|----|--|--------------|
| 2. | D+H+F+L+T | Eq. (3.2.9) |
| 3. | $D + H + F + (L_r or R)$ | Eq. (3.2.10) |
| 4. | $D + H + F + 0.75(L + T) + 0.75(L_r \text{ or } R)$ | Eq. (3.2.11) |
| 5. | D + H + F + (W or 0.7E) | Eq. (3.2.12) |
| 6. | D + H + F + 0.75(W or 0.7E) + 0.75L + 0.75 (L _r or R) | Eq. (3.2.13) |
| 7. | 0.6D + W + H | Eq. (3.2.14) |
| 8. | 0.6D + 0.7E + H | Eq. (3.2.15) |



Earthquake Design Flow Chart



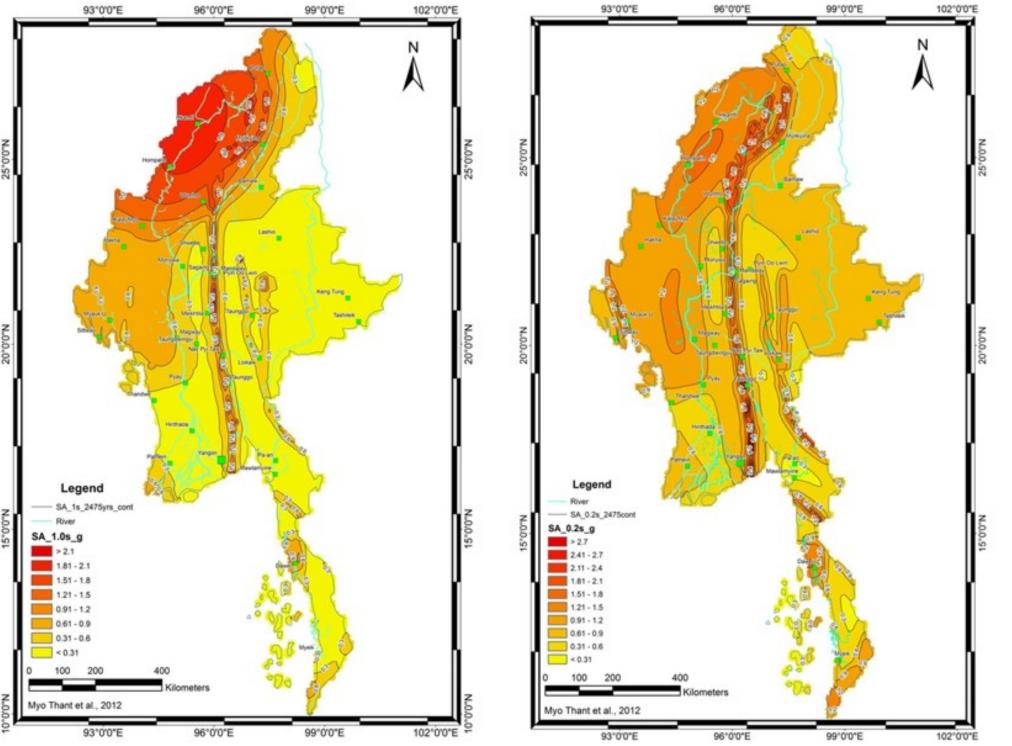






- Ground Acceleration, S_s, S₁
- Spectral Response Acceleration at 2% Probability in 50 Years with 5% Critical

Damping, Site Class B



S_s for 0.2 second Short Period Acceleration for Short Buildings

S₁ for 1 second Long Period Acceleration for Tall Buildings



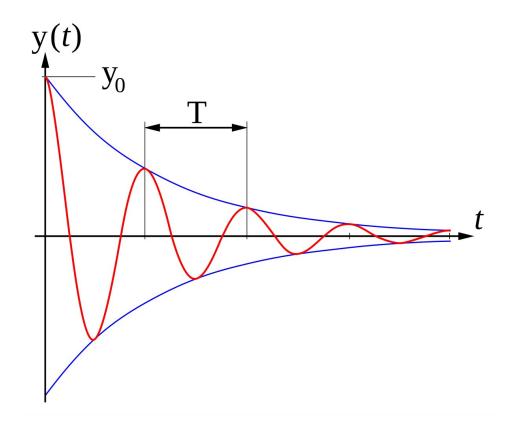
• Ground Acceleration, S_s, S₁ • Ground Acceleration, S_s, S₁

- Spectral Response Acceleration at 2% Probability in 50 Years with 5% Critical
 Spectral Response Acceleration at 2% Probability in 50 Years with 5% Critical Damping, Site Class B Damping, Site Class B
- Under the assumption of a Poisson distribution, seismic risk, expressed in terms
- Underptbeassitympteanofakeiseanedistributionifædsmigniskdexpressediven texas strational imithothe are a reaser by the service of interval entire terms and the service of the service
- a given exposure time (t) with the average recurrence interval (R) can be $p=1-e^{-R}$ expressed as:
- For 50 years design life, 2% probability gives $\mathbf{R} = \mathbf{2475}$ years
- For 50 years design life, 2% probability gives **R = 2475 years**

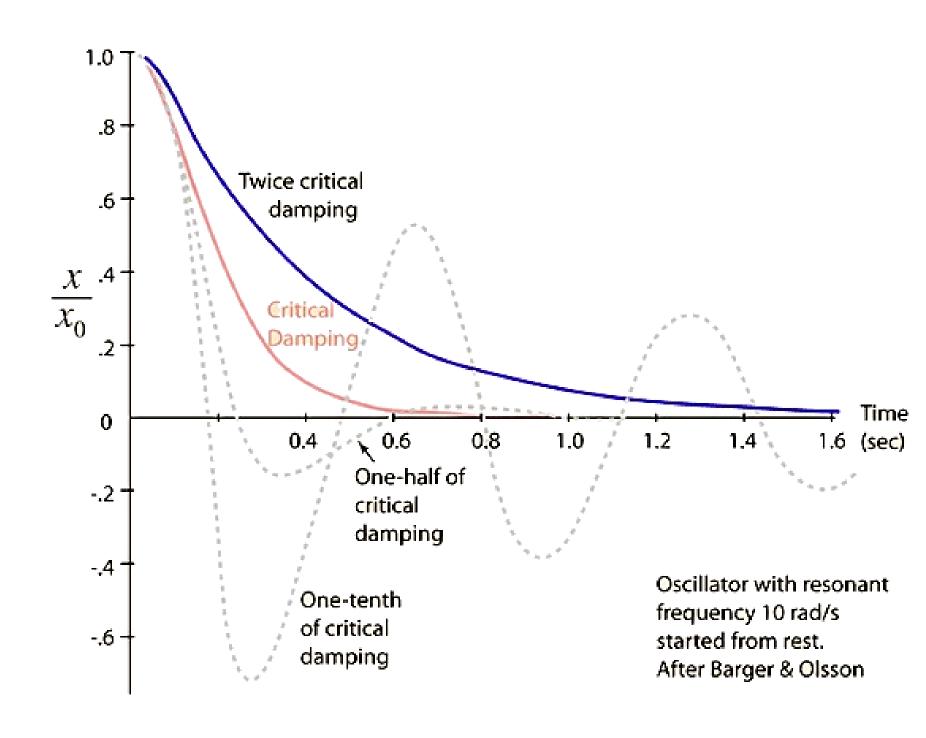


Ground Acceleration, S_s, S₁

Spectral Response Acceleration at 2% Probability in 50 Years with 5% Critical Damping, Site Class B



- For buildings,
- oscillation decays at
- 5% critical damping.



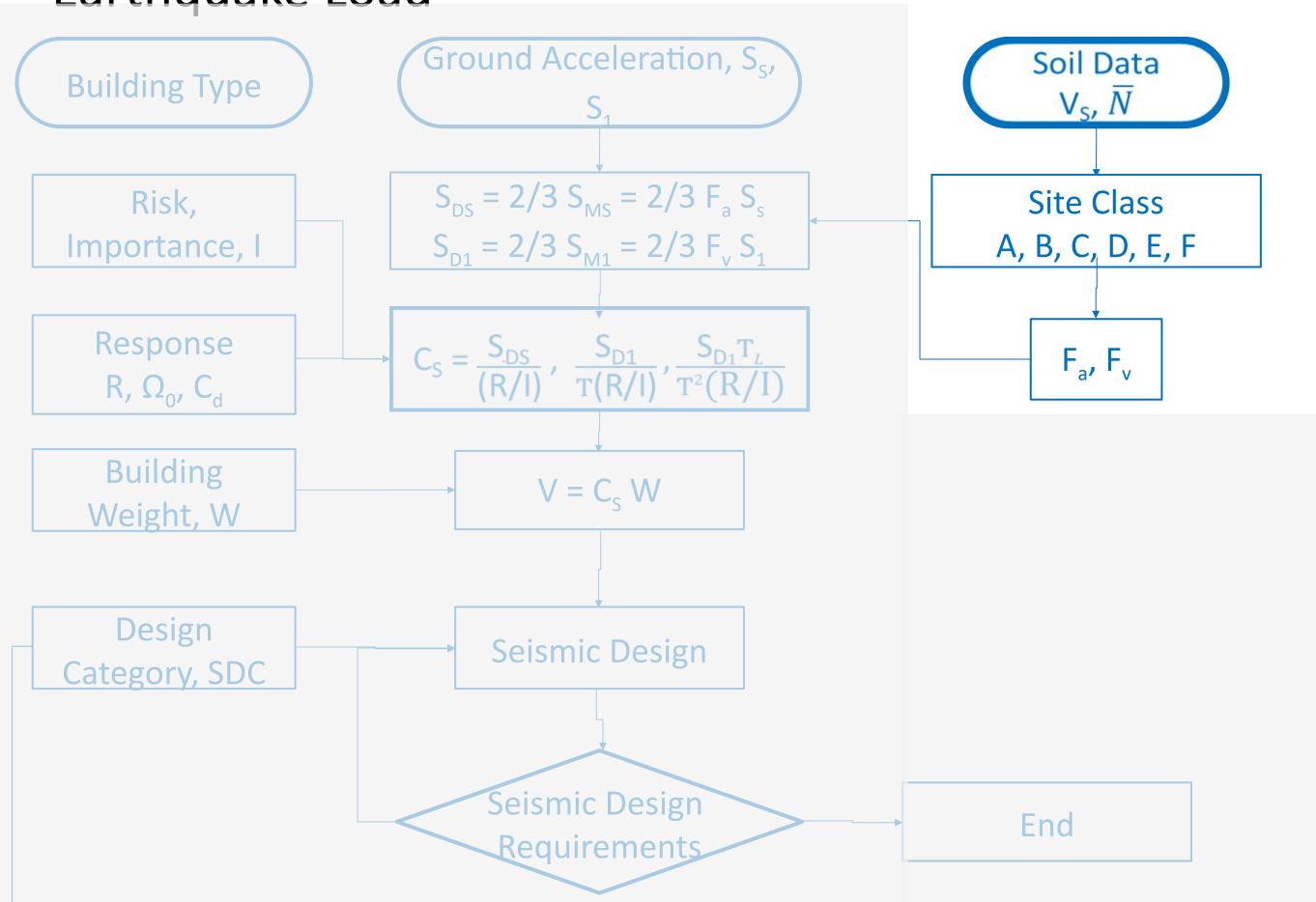


Site Class

Spectral Response Acceleration at 2% Probability in 50 Years with 5% Critical Damping, Site Class B

| SITE CLASS | SOIL PROFILE NAME | AVERAGE PROPERTIES IN TOP 100 feet | | | |
|------------|-------------------------------|--|-------------|---|--|
| | | Shear wave velocity, v _s , (ft/s) | SPT, N | Undrained shear, s _u , (psf) | |
| А | Hard rock | v s > 5,000 | N/A | N/A | |
| В | Rock | $2,500 > v_s > 5,000$ | N/A | N/A | |
| С | Very dense soil and soft rock | $1,200 \le v_s \le 2,500$ | N > 50 | s _u ≤ 2,000 | |
| D | Stiff soil profile | $600 \le v_s \le 1,200$ | 15 ≤ N ≤ 50 | $1,000 \le s_u \le 2,000$ | |
| E | Soft soil profile | v s < 600 | N < 15 | s _u < 1,000 | |
| F | Very soft / Unknown | Need further investigation. | | | |







Site Coefficients F_a, F_v

Fa: Site modification factor

for short period,

Short Buildings

Fv: Site modification factor for long period,

Tall Buildings

TABLE 3.4.3 SITE COEFFICIENT, F_a

| | | | xımum Conside Acceleration Pai | - | - | |
|------------|-------------------------------|-----------------------------|-----------------------------------|-----------------------------|-----------------------|--|
| Site Class | <i>S</i> _{\$} ≤ 0.25 | <i>S</i> ₅ = 0.5 | <i>S</i> ₅ = 0.75 | <i>S</i> _S = 1.0 | S _S ≥ 1.25 | |
| А | 0.8 | 0.8 | 0.8 | 0.8 | 0.8 | |
| В | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | |
| С | 1.2 | 1.2 | 1.1 | 1.0 | 1.0 | |
| D | 1.6 | 1.4 | 1.2 | 1.1 | 1.0 | |
| E | 2.5 | 1.7 | 1.2 | 0.9 | 0.9 | |
| F | See Section 11.3.4.7 | | | | | |

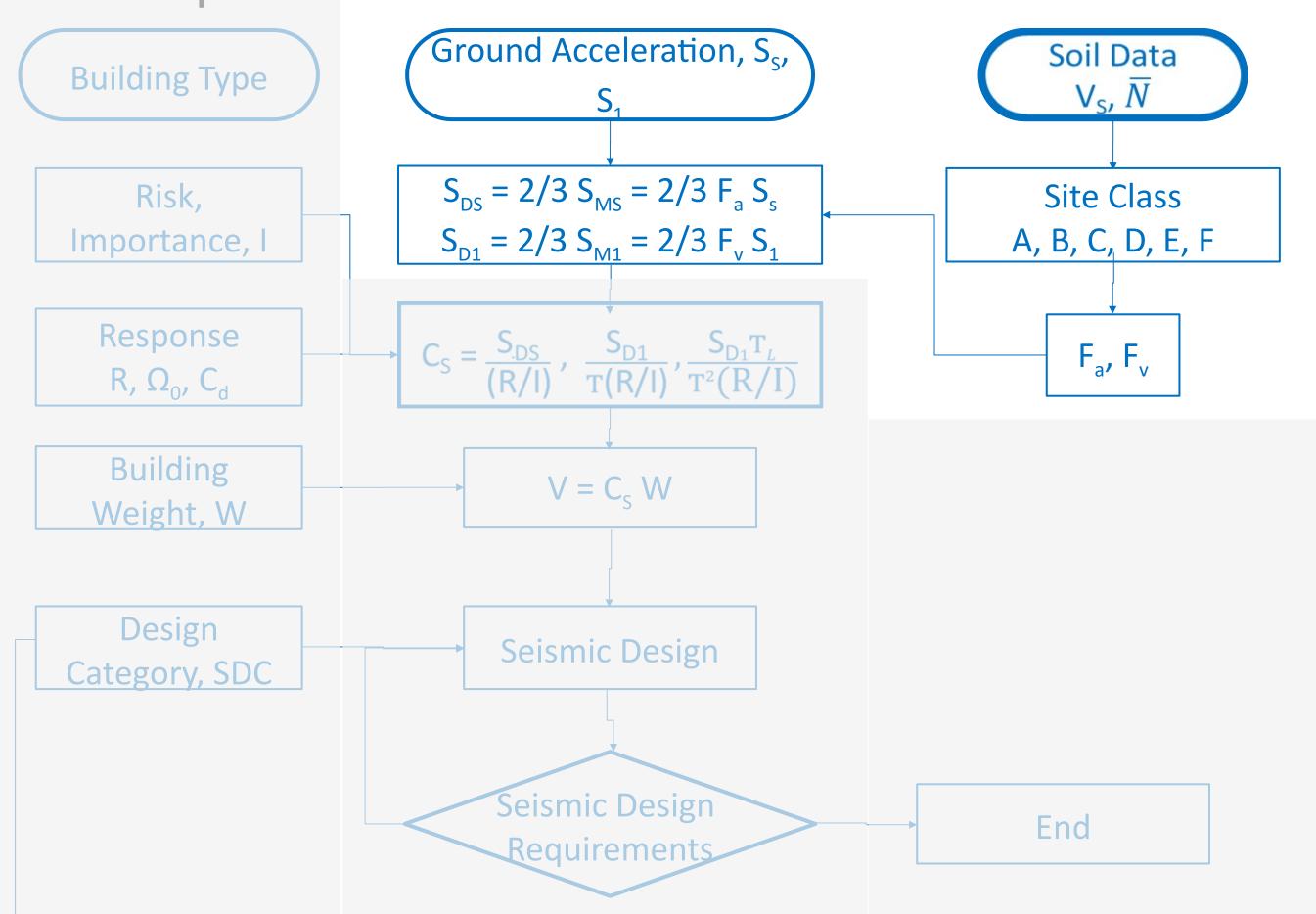
NOTE: Use straight-line interpolation for intermediate values of Ss.

TABLE 3.4.4 SITE COEFFICIENT, F_{ν}

| Site Class | Mapped Maximum Considered Earthquake Spectral Response Acceleration Parameter at 1-s Period | | | | | | |
|------------|--|--|-----|-------------|---------------|--|--|
| | $s_1 \leq 0.1$ | $s_1 \leq 0.1$ $s_1 = 0.2$ $s_1 = 0.2$ | | $S_1 = 0.4$ | $s_1 \ge 0.5$ | | |
| A | 0.8 | 0.8 | 0.8 | 0.8 | 0.8 | | |
| В | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | | |
| С | 1.7 | 1.6 | 1.5 | 1.4 | 1.3 | | |
| D | 2.4 | 2.0 | 1.8 | 1.6 | 1.5 | | |
| E | 3.5 | 3.2 | 2.8 | 2.4 | 2.4 | | |
| F | See Section 11.3.4.7 | | | | | | |

NOTE: Use straight-line interpolation for intermediate values of S_1 .



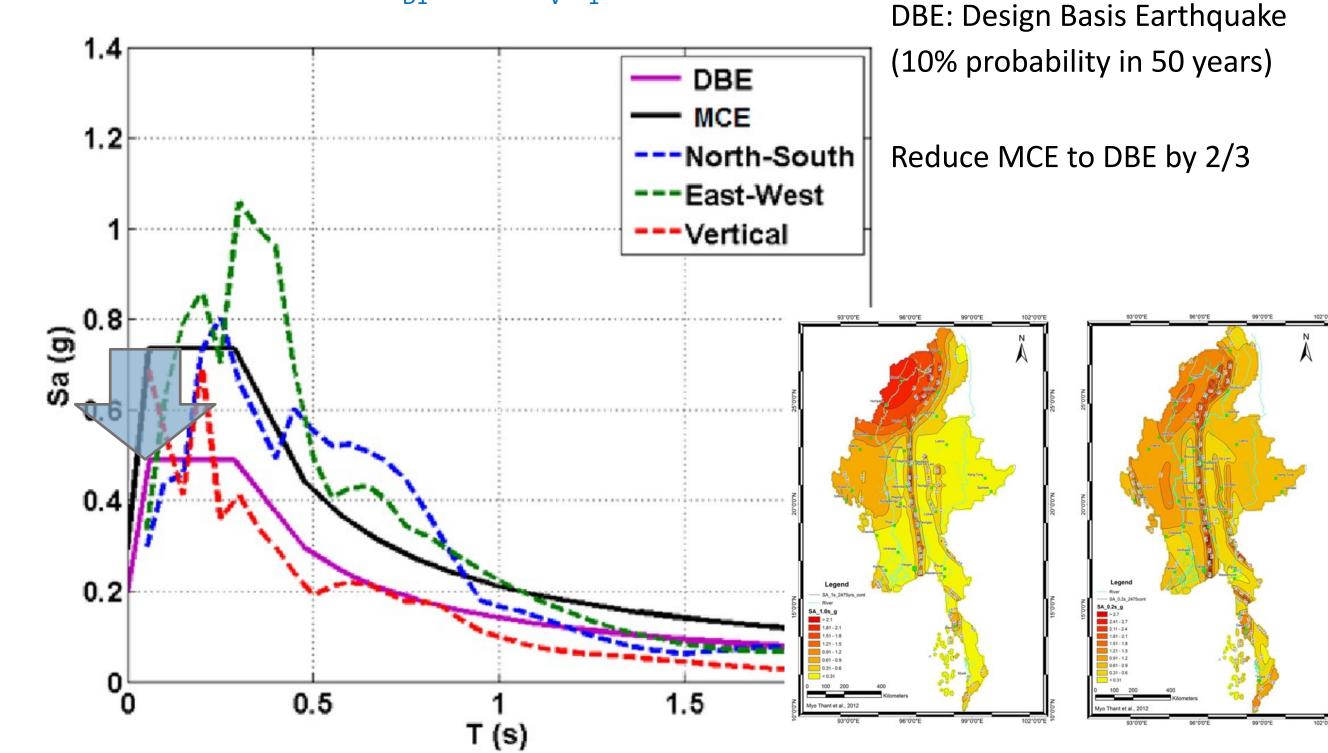




Design Spectral Acceleration, S_{DS}, S_{D1}

• SDS = 2/3 Fa Ss

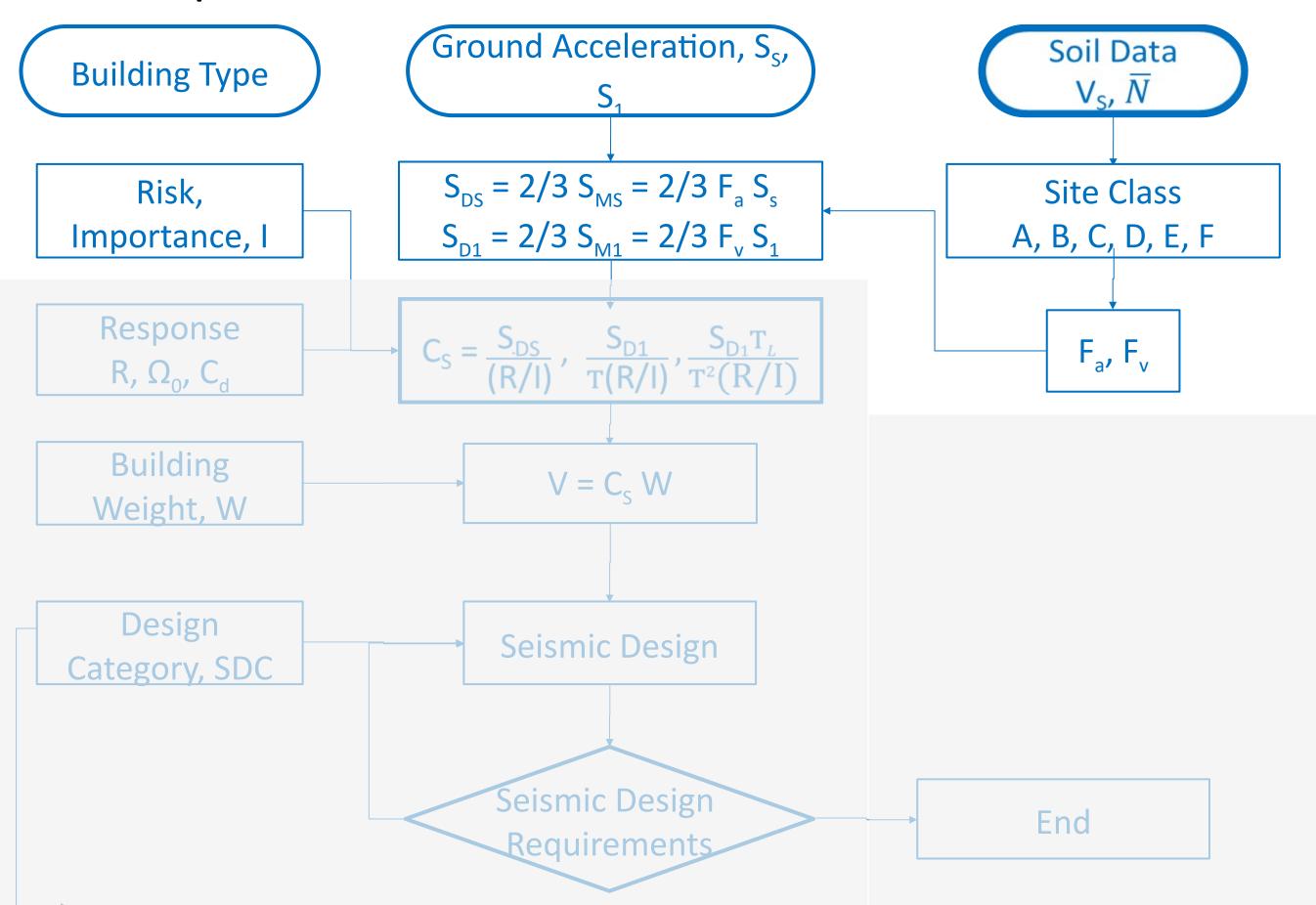
•
$$S_{D1} = 2/3 F_{v} S_{1}$$



MCE: Maximum Considered Earthquake

(2% probability in 50 years)







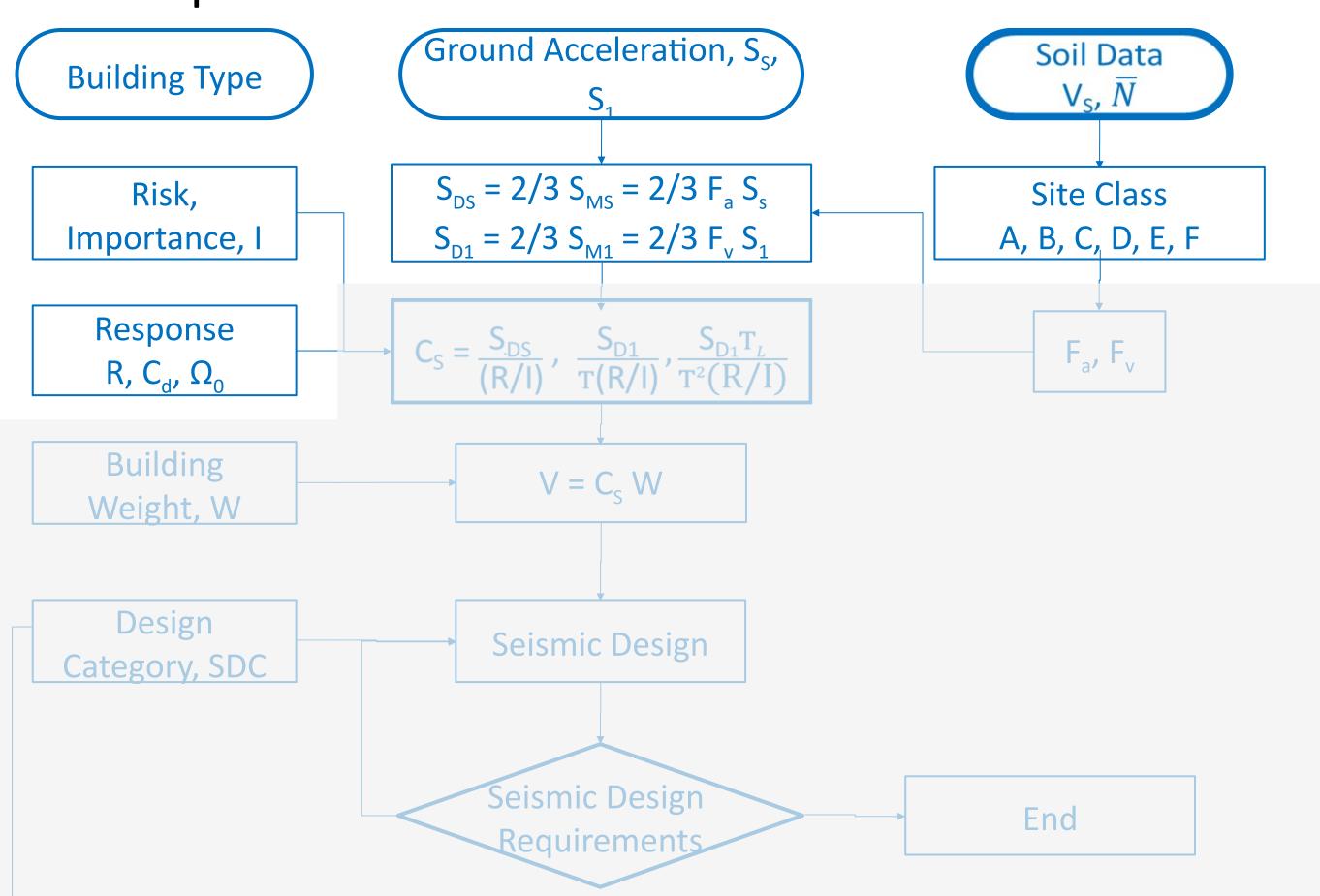
• Importance Factor, I

| Risk Category | Nature of Occupancy |
|---------------|--|
| 1 | Representing a low hazard to human life in the event of failure |
| П | Except those listed in other categories |
| III | Represent a substantial hazard to human life in the event of failure |
| IV | Designed as essential facilities |

TABLE 3.4.6 IMPORTANCE FACTORS

| Occupancy Category | I |
|--------------------|------|
| I or II | 1.0 |
| III | 1.25 |
| IV | 1.5 |



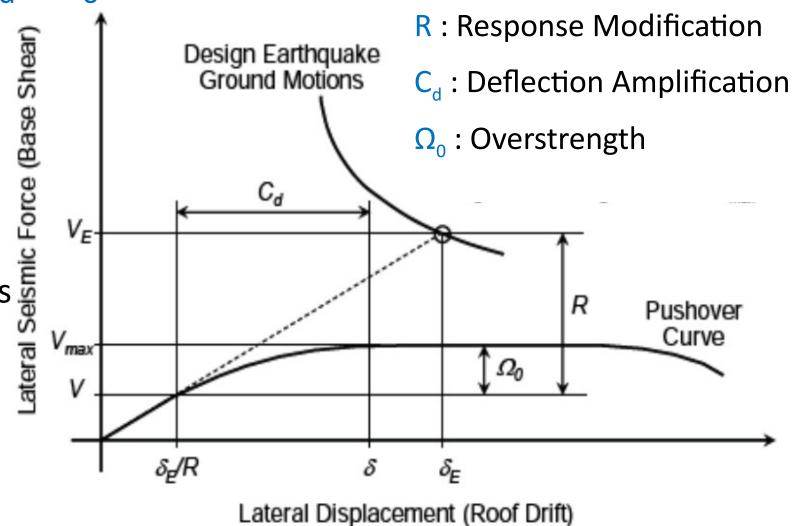




Response Factors: R, C_d , Ω_0

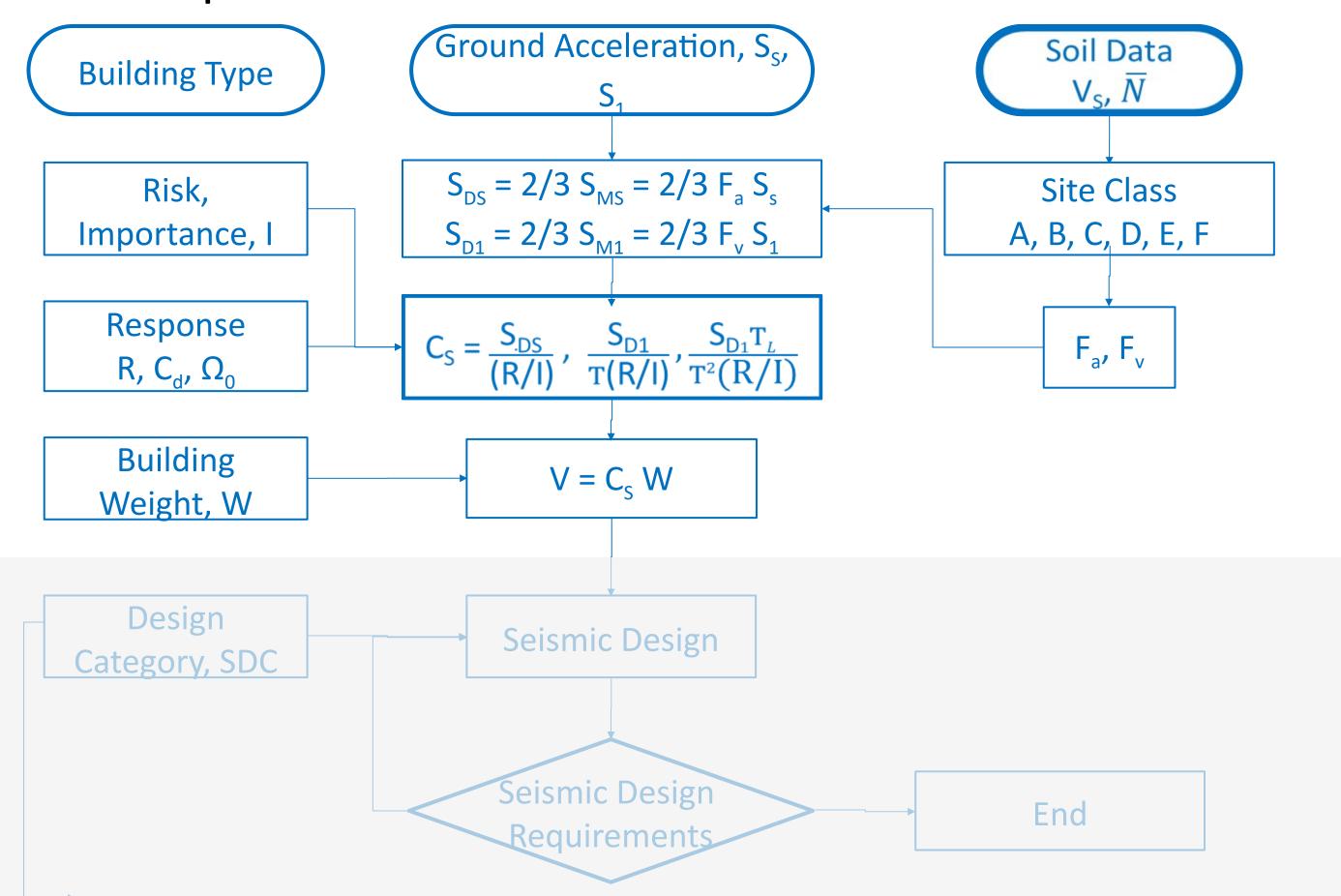
Seismic Force Resisting Systems

- A Bearing Wall Systems
- B Building Frame Systems
- C Moment Resisting Frame Systems
- D Dual Systems with SMRS
- E Dual Systems with IMRS
- •F Shear Wall Frame Interactive Systems
- G Cantilever Column Systems
- H Steel Systems



| Seismic Force-Resisting System | ASCE 7-05, Required Detailing | Response Modification Factor, R* | System Over strength Factor, Ω _b # | Deflection Amplification Factor, C _d ^b | Structural System Limitations and Building Height (ft) Limit ^c Seismic Design Category | | | | |
|--|-------------------------------------|--|---|--|---|--------|-----------------|-----------------|-----------------|
| System | ASK Re De | Re: Aod Fac | : sk tr me | Def mp | | Seismi | c Desig | n Catego | ory |
| | | _ | 5 | A | В | С | D ^d | Ed | Fe |
| A. BEARING WALL SYSTEMS | | | | | | | | | |
| 1.Special reinforced concrete shear walls | 14.2 | 5 | 21/2 | 5 | NL | NL | 160 | 160 | 100 |
| 2.Ordinary reinforced concrete shear walls | 14.2 | 4 | 21/2 | 4 | NL | NL | NP | NP | NP |
| 3. Detailed plain concrete shear walls | 14.2 | 2 | 21/2 | 2 | NL | NP | NP | NP | NP |
| 4. Ordinary plain concrete shear walls | 14.2 | 11/2 | 21/2 | 11/2 | NL | NP | NP | NP | NP |
| 5.Intermediate precast shear walls | 14.2 | 4 | 214 | 4 | NL | NL | 40 ^k | 40 ^k | 40 ^k |
| 6. Ordinary precast shear walls | 14.2 | м | 21/2 | м | NL | NP | NP | NP | NP |
| 7. Special reinforced masonry shear walls | 14.4 | 5 | 21/2 | 31/2 | NL | NL | 160 | 160 | 100 |
| 8. Intermediate reinforced masonry shear walls | 14.4 | 31/2 | 21/2 | 21/4 | NL | NL | NP | NP | NP |



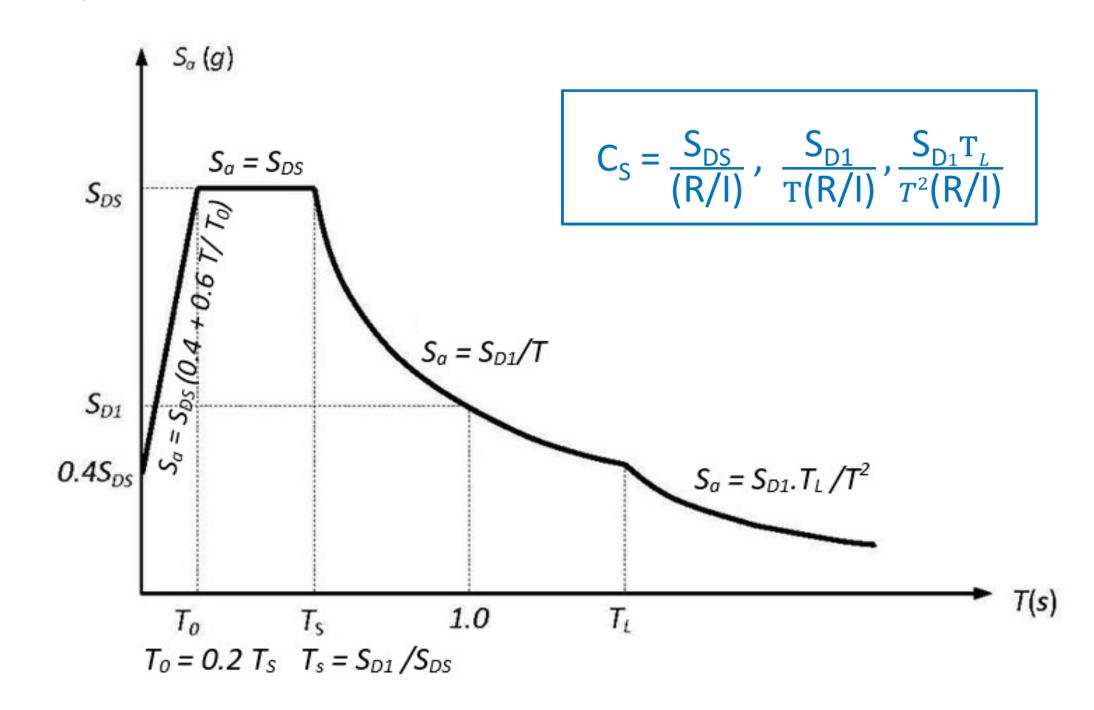




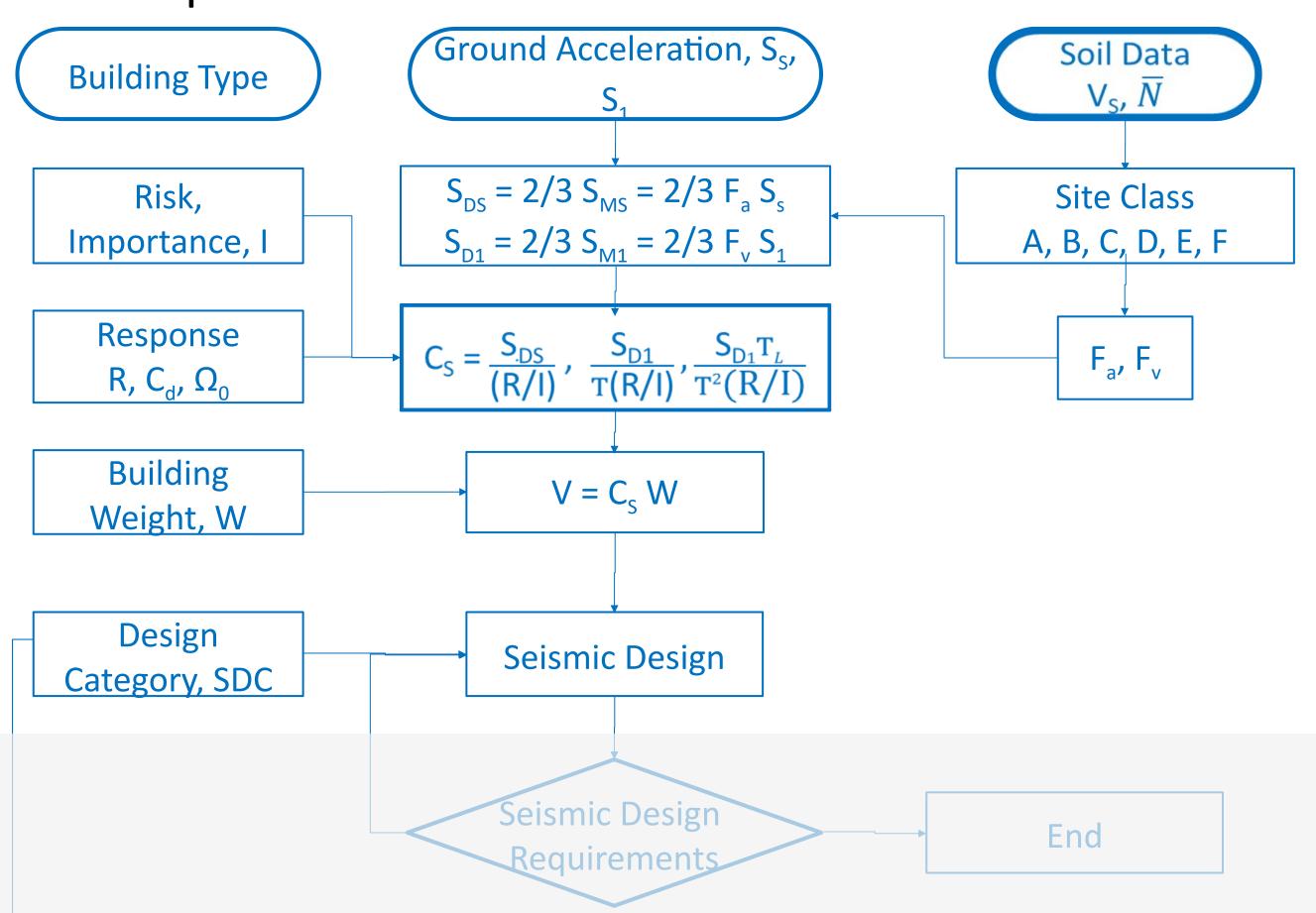
Seismic Base Shear: V

•
$$V = C_S W$$

• C_s: Response Coefficient









- Seismic Design Category, SDC
- Larger Seismicity needs Higher SDC

SEISMIC DESIGN CATEGORY (SDC)

There are various correlations of the qualitative Modified Mercalli Intensity (MMI) with quantitative characterizations of ground-shaking limits for the various SDCs.

| MMI V | No real damage | SDC A | |
|----------|--|-------|--|
| MMI VI | Light nonstructural damage | SDC B | |
| MMI VII | Hazardous nonstructural damage | SDC C | |
| MMI VIII | Hazardous damage to susceptible structures | SDC D | |
| MMI IX | Hazardous damage to robust structures | SDC E | |



Seismic Design Category, SDC

More Important (Higher Occupancy Risk) needs
 Higher SDC

| Risk Category | Nature of Occupancy |
|---------------|--|
| I | Representing a low hazard to human life in the event of failure |
| II | Except those listed in other categories |
| III | Represent a substantial hazard to human life in the event of failure |
| IV | Designed as essential facilities |

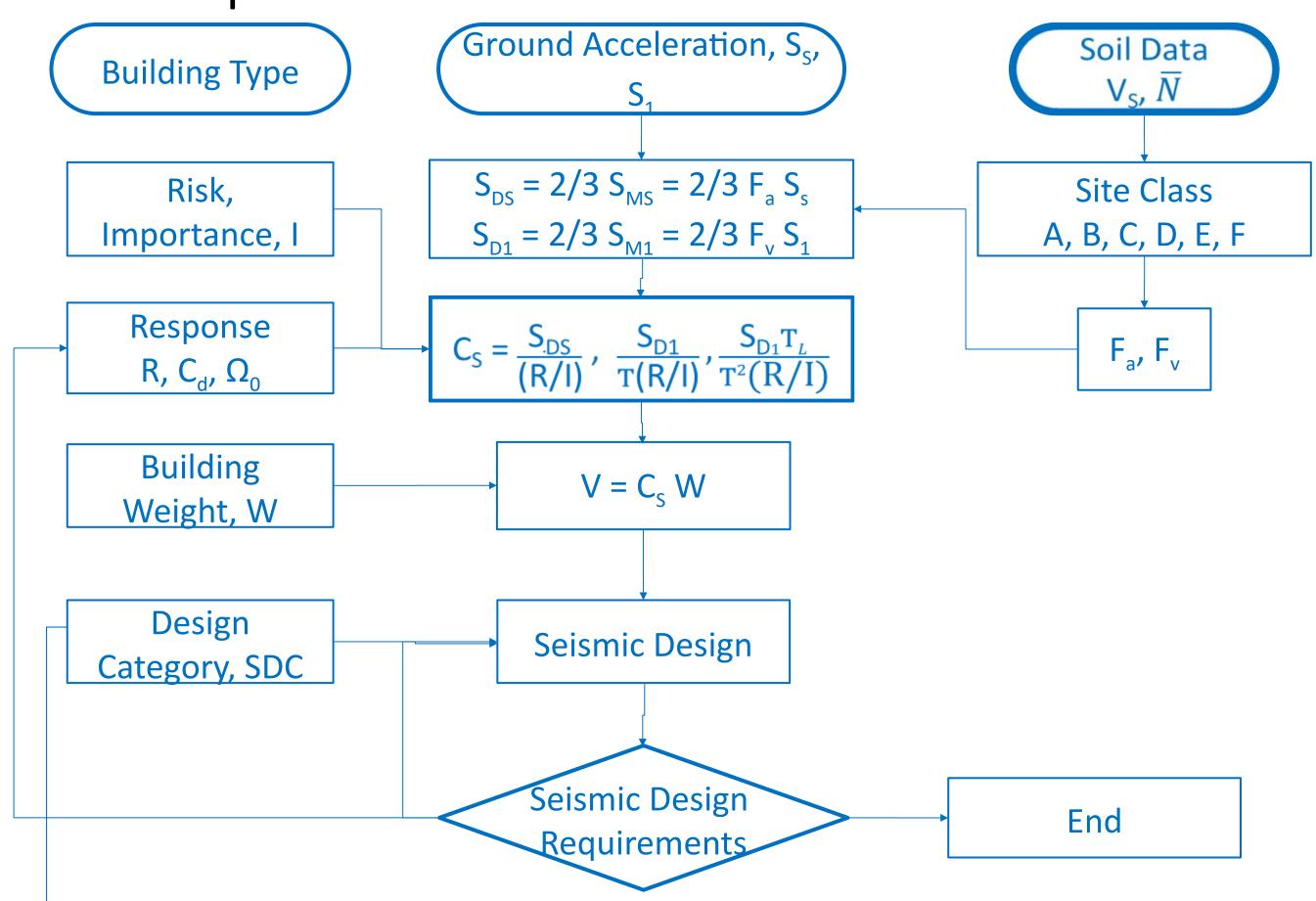


Seismic Design Category, SDC

SDC with Seismicity .vs. Occupancy Risk

| S _{DS} | S _{D1} | Level of Seismicity | 1 & 11 | III | IV |
|-----------------|------------------|------------------------|-----------|-----|----|
| <0.167 g | <0.067 g | Very Low | Α | Α | Α |
| 0.167 to 0.33 g | 0.067 to 0.133 g | Low | Α | В | С |
| 0.33 to 0.5 g | 0.133 to 0.2 g | Moderate | В | С | D |
| >0.5 g | >0.2 g | High | С | D | D |
| | >0.75 g | Severe | D | E | F |







Seismic Design Requirements according to SDC

| SDC | Building Type and Expected MMI | Seismic Criteria | | | | | | | |
|-----|--|---|--|--|--|--|--|--|--|
| Α | Buildings located in regions hav- ing a very small probability of experiencing damaging earth- quake effects | No specific seismic design requirements but structures are required to have complete lateral-force-resisting systems and to meet basic structural integrity criteria. | | | | | | | |
| В | Structures of ordinary occupancy that could experience moderate (MMI VI) intensity shaking | Structures must be designed to resist seismic forces. | | | | | | | |
| С | Structures of ordinary occupancy that could experience strong (MMI VII) and important structures that could experience moderate (MMI VI) shaking | Structures must be designed to resist seismic forces. Critical nonstructural components must be provided with seismic restraint. | | | | | | | |
| D | Structures of ordinary occupancy that could experience very strong shaking (MMI VIII) and important structures that could experience MMI VII shaking | Structures must be designed to resist seismic forces. Only structural systems capable of providing good performance are permitted. Nonstructural components that could cause injury must be provided with seismic restraint. Nonstructural systems required for life safety protection must be demonstrated to be capable of post-earthquake functionality. Special construction quality assurance measures are required. | | | | | | | |



Seismic Design Requirements according to SDC

| Building Type and Expected MMI | Seismic Criteria | | | | | |
|-----------------------------------|---|--|--|--|--|--|
| Structures of ordinary occupancy | Structures must be designed to resist seismic forces. | | | | | |
| of major active faults capable of | Only structural systems that are capable of providing superior performance permitted. | | | | | |
| shaking | Many types of irregularities are prohibited. | | | | | |
| | Nonstructural components that could cause injury must be provided with seismic restraint. | | | | | |
| | Nonstructural systems required for life safety protection must be demonstrated to be capable of post-earthquake functionality. | | | | | |
| | Special construction quality assurance measures are required. | | | | | |
| | | | | | | |
| Critically important structures | Structures must be designed to resist seismic forces. | | | | | |
| of major active faults capable of | Only structural systems capable of providing superior performance permitted are permitted. | | | | | |
| shaking | Many types of irregularities are prohibited. | | | | | |
| | Nonstructural components that could cause injury must be provided with seismic restraint. | | | | | |
| | Nonstructural systems required for facility function must be demonstrated to be capable of postearthquake functionality. | | | | | |
| | Special construction quality assurance measures are required. Additional construction and an expecial construction quality assurance measures Additional construction and assurance measures are required. | | | | | |
| | Structures of ordinary occupancy located within a few kilometers of major active faults capable of producing MMI IX or more intense shaking Critically important structures located within a few kilometers of major active faults capable of producing MMI IX or more intense | | | | | |



Seismic Design Requirements for Cracked Sections

Table 6.6.3.1.1(a)—Moment of inertia and crosssectional area permitted for elastic analysis at factored load level

| Member and condition Columns | | Moment of Inertia | Cross-sectional area | | |
|-------------------------------------|-----------|----------------------|-------------------------|--|--|
| | | 0.70I _E | | | |
| Walls | Uncracked | 0.70I _g | | | |
| watts | Cracked | 0.35I _g | 1.04s | | |
| Beams Flat plates and flat slabs | | 0.35I _E | | | |
| | | 0.25Ig | | | |



Seismic Design Requirements Permitted Analysis Procedures

TABLE 12.6-1 PERMITTED ANALYTICAL PROCEDURES

| | 12.6-1 PERMITTED ANALY | IVALI | IOOLDO | |
|-------------------------------|--|--|---|--|
| Seismic Design Category | Structural Characteristics | Equivalent Lateral Force Analysis Section 12.8 | Modal Response Spectrum Analysis Section 12.9 | Seismic Response History Procedures Chapter 16 |
| B, C | Occupancy Category I or II buildings of light-framed construction not exceeding 3 stories in height | P | P | P |
| | Other Occupancy Category I or II buildings not exceeding 2 stories in height | P | P | P |
| 1 | All other structures | P | P | P |
| D, E, F | Occupancy Category I or II buildings of light-framed construction not exceeding 3 stories in height | P | P | Р |
| | Other Occupancy Category I or II buildings not exceeding 2 stories in height | P | P | P |
| | Regular structures with $T < 3.5T_s$ and all structures of light frame construction | P | P | Р |
| | Irregular structures with $T < 3.5T_s$ and having only horizontal irregularities Type 2, 3, 4, or 5 of Table 12.2-1 or vertical irregularities Type 4, 5a, or 5b of Table 12.3-1 | P | P | P |
| | All other structures | NP | P | P |



Seismic Design Requirements for Horizontal Irregularity

| Cat | Cat. Sr. Type | | | SDC | | | | | Remedial | | | | | | Remedial Measures | |
|------------|---------------|----------------------------|--|-----|-----|---|-----|---------|----------|---|-----|---|-------------|-----|-------------------|---|
| Cat. | 31. | Туре | | Α | ВС | D | E | F | 1 | 2 | 3 4 | 5 | 6 | 7 8 | 3 9 | Source: ASCE 7-05, Table 12.3.1 and 12.3.2. |
| | 1 | Torsional | 1a. Moderate: $\Delta 1 > 1.2 \Delta a vg$ 1b. Extreme: $\Delta 1 > 1.4 \Delta a vg$ $\Delta a vg = (\Delta 1 + \Delta 2)/2$ | | 0 | | Х | X X X X | | 0 | 0 0 | | X X X | | | Increase design forces determined by static procedure by Some connection of diaphragms to vertical elements and to collectors, and for connections of collectors to the vertical elements. Collectors and their connections also shall be |
| Horizontal | 2 | Reentrant Corner | B > 15% A D > 15% C E > 15% C | | | 0 | C | 0 | Ο | | | О | | | | designed for these increased forces unless they are designed for load combinations with overstrength factor Ω o (ASCE 7-05, Section 12.3.3.4). 2. Multiply, M_{ta} , the torsional moment due to accidental |
| | 3 | Diaphragm Discontinuity | Area XY > 50% AB | | | o | c | 0 | Ο | | | О | | | | torsion by a torsional amplification A_x (ASCE 7-05, Section 12.8.4.3). $A_x = (\Delta_{max}/1.2\Delta_{avg})^2 \le 3$ |
| | 4 | Out of Plane Offsets | Out of Plane Offset | | 0 0 | |) C | 0 | | 0 | 0 | 0 | | 0 | | 3. Perform a 3D dynamic analysis with due consideration for diaphragm stiffness. Use cracked section properties for concrete elements. Include P Δ effects (ASCE 7-05, Section 12.7.3). |
| | 5 | Nonparallel | Nonparallel System | | 0 | |) C | 0 0 | | | 0 | 0 | | | | 4. Compute story drift, Δ , as the largest difference of the deflections along any of the edges of the structure at the top and bottom of the story under consideration (ASCE 7-05, |
| | 1 | Stiffness | 1a. Moderate: A <70% B or A < 80% (B+C+D)/3 1b. Extreme: A <60% B or A < 70% (B+C+D)/3 | | | 0 | × | × | | | | О | x | | | Section 12.12.1). 5. Use model analysis or more rigorous procedure (ASCE 7-05, Table 12.6.1). 6. Not permitted (NP) (ASCE 7-05, Section 12.3.3.1). |
| | 2 | Weight | Mass B > 150% Mass A | | | 0 | C | 0 | | | T | О | | | Ī | 7. Design columns, beams, trusses, or stabs supporting discontinuous walls or frames to resist maximum axial forces determined by using the following load combinations with the overstrength factor Ω_o : |
| Vertical | 3 | Vertical Geometry | X > 130% Y | | | 0 | C | 0 | | | Ī | О | | | | $(1.2 + 0.2S_{DS})D + \Omega_oQ_E + L + 0.2S$ $(0.9 - 0.2S_{DS})D + \Omega_oQ_E + 1.6H$ (ASCE 7-05, Section 12.3.3.3). 8. Use $100\% \times 30\% $ y, if you are using ELF or modal analysis. |
| | 4 | Inplane Discontinuity | L1 > L | | 0 0 | |) C | 0 | 0 | | - | О | | 0 | | Use simultaneous application of load, if you are analyzing the structure using a linear or nonlinear response history procedure (ASCE 7-05, Section 12.5.3). 9. Maximum height limit 30 ft or two stories, unless the weak |
| | 5 | Weak Story | 5a. Moderate: A < 80% B 5b. Extreme: A < 65% B | | 9 9 | 0 | × | × | | | | О | x | | | story is capable of resisting a seismic force = Ω 0 times the design force (ASCE 7-05, Section 12.3.3.2). |

Ref: 1. ASCE 7-05- Table 12.3.1./ 2. Reinforced Concrete Design of Tall Buildings, Taranath, Table 5.23, 5.24

- 1. Increase 25%
- 2. Torsion
- 3. Dynamic
- 4. Drift
- 5. Detail Analysis
 - 6. Not Permitted
- 7. Overstrength
- 8. Directional
- 9. Height Limit



Seismic Design Requirements for Vertical Irregularity

| | | | to - Posside Potent At 1 to 4 and 400 | | 8 5 | | 5 5 | | <u> </u> | SE | TEI | ¥E | <u>.</u> | | Seures Measures Seures Measures Meas | ı | |
|-----------|-----|---------------------|--|----------------|---------------|---|-------------------------|----------------|----------|-----------|---------------------|--------------|----------|--|--|---------------------------------|---------------------|
| _ | - | Tersional Espain | | | | 0 | 0 0 | . 0 | 00 | 3 8 | 0 | ** | | | dellesaer Land for describe of et es les er a leite arme et et. Lange es et for grape introducer a social ser et es et et e. Lightly of a signification of the est est est est est est est est est es | | |
| _ | - | Sizebrator. | mar or mana merant | # | 8 8 | 8 | 8 6 | | = : | | 8 | : | 3 | | a: Perferm a 35 dynamic analysis with due consideration for disphragm swiffness. Use cracked seven properties for consequents include 24 effects (ASCE 7-05, Section | | |
| _ | - | Servers Servers | A CONTRACTOR OF THE | | | 0 0 | ~ ~ | e . | 1 | 3 | 0 | >< | - | | A Carrients are to diffy, by as the largest difference of the tent | | |
| _ | | Weight Weight | | + | | - | | 3 | | + | 0 | | + | | disconfinance and in the means, relative and means and the real terms of the contract of the c | | |
| | 1 | Aveat Stary | na - readerate : A - Austria | | 0 0 | 8 | | 2 6 | | | 8 | | 3 | - | ET I I III I ET ET EN EN I I I I I I I I I I I I I I I I I | | |
| Cat. | Sr. | Туре | | | SDC | | | | | | emedial | | | | Remedial Measures | | |
| | | | | | ВС | | | | | | + | _ | 7 8 | 9 | Source: ASCE 7-05, Table 12.3.1 and 12.3.2. | | |
| | 1 | | 1a. Moderate: Δ1 > 1.2 Δav | ; <u> </u> | 0 | | X X | | | | | X | | | 1. Increase design forces determined by static procedure by | | 4 |
| | | Torsional | 1b. Extreme: $\Delta 1 > 1.4 \Delta a vg$ | | 0 | | XX | | O | | 기 | X | | | 25% for connection of diaphragms to vertical elements and to | • | 1. Increase 25% |
| | | | $\Delta a vg = (\Delta 1 + \Delta 2)/2$ | | | О | XX | 0 | 0 | olc | 0 | X | | | collectors, and for connections of collectors to the vertical elements. Collectors and their connections also shall be | | |
| | | | D 450/ 1 | | | Н | | \Box | | | Н | | + | | | | |
| | 2 | Reentrant | B > 15% A | | | اما | ماد | | | | | | | | designed for these increased forces unless they are designed for load combinations with overstrength factor Ω o (ASCE 7-05, | • | 2. Torsion |
| / | | Corner | D > 15% C | | | U | ٩Ľ | | | | | | | | Section 12.3.3.4). | | 2. 10131011 |
| orizontal | | | E > 15% C | | | | | ш | | | | | | | | | |
| | 3 | Diaphragm | | | | | | П | | | | | | | 2. Multiply, M _{ta} , the torsional moment due to accidental | | |
| | | | Area XY > 50% AB | | | lolo | old | olol | | | lol | | | | torsion by a torsional amplification A_x (ASCE 7-05, Section | • | 3. Dynamic |
| 0 | | Discontinuity | | | | ш | | 12.8.4.3). | | | | | | | | | |
| 工 | | 22.5 | | + | | ш | | н | | | ш | | | | $A_{x} = (\Delta_{\text{max}}/1.2\Delta_{\text{avg}})^{2} \le 3$ | | |
| | 4 | Out of Plane | | 1 | o o | | | | | | | | 기 | | 3. Perform a 3D dynamic analysis with due consideration for | | 1 D.::ft |
| | | Offsets | Out of Plane Offset | | | | - | | | | | | | + | diaphragm stiffness. Use cracked section properties for | • | 4. Drift |
| | | | J ▋ ■ | | | | \circ | 7 | 0 | | | ١ | 기 | | concrete elements. Include P Δ effects (ASCE 7-05, Section 12.7.3). | | |
| | 5 | | | | o | | | | | 0 | | | | | .7.3). Compute story drift, Δ, as the largest difference of the | | |
| | | Nonparallel | Nonparallel System | | О | | | 11 | | | | | С | | deflections along any of the edges of the structure at the top | • | 5. Detail Analysis |
| | | l | non-paramet 5 yetem | | _ | | 00 | + | | | 0 | + | C | | and bottom of the story under consideration (ASCE 7-05, | ` | 3. Detail / marysis |
| | | | 1- M-d | \blacksquare | | | | 4 | | 4 | 19 | | | 4 | Section 12.12.1). | | |
| | 1 | | 1a. Moderate: A <70% B or A < 80% (B+C+D)/3 | | | ш | | | | | ш | | | | 5. Use model analysis or more rigorous procedure (ASCE 7-05, | | |
| | | Stiffness | 1b. Extreme: A <60% B or | | | | $\langle \ \ \ $ | | | | X | | | Table 12.6.1). | • | Not Permitted | |
| | | | A < 70% (B+C+D)/3 | | | ш | | | | | ш | | | | 6. Not permitted (NP) (ASCE 7-05, Section 12.3.3.1). | | |
| | | | (- n - | | | ш | | | | + | Н | | + | | 7. Design columns, beams, trusses, or stabs supporting | | |
| | 2 | Weight | NA D > 4500/ NA A | | | اما | olo | | | | | | | | discontinuous walls or frames to resist maximum axial forces | • | 7. Overstrength |
| | | vveigiit | Mass B > 150% Mass A | | | $I^{U}I$ | Y۱ | 11 | | | \mathbb{I}^{\vee} | | | | determined by using the following load combinations with the | | 7. Overstrength |
| _ | | | | | | | | | | | | | | | verstrength factor Ω_o : | | |
| ertical | | Vortical | | | | ΙĪ | | | | | | | | | $(1.2 + 0.2S_{DS})D + \Omega_oQ_E + L + 0.2S$ | | |
| rti | 3 | Vertical | X > 130% Y | | | Ю | o | | | | | | | | $(0.9 - 0.2S_{DS})D + \Omega_oQ_E + 1.6H$ (ASCE 7-05, Section 12.3.3.3). | • | 8. Directional |
| V e | | Geometry | | | | | | | | | | | | 8. Use 100% x + 30% y, if you are using ELF or modal analysis. | | | |
| | | | | | | | | \blacksquare | - | + | | | | | Use simultaneous application of load, if you are analyzing the | | |
| | 4 | Inplane | | | $\circ \circ$ | 이 | | | | | | | 이 | | | | 0 Hoight Limit |
| | | Discontinuity | L1 > L | | | | | | | | | | | | procedure (ASCE 7-05, Section 12.5.3). | | 9. Height Limit |
| | | 1 | | | | $ \circ $ | olc | О | | | | C | | | 9. Maximum height limit 30 ft or two stories, unless the weak | | |
| | | | | | | | | | | | П | | | Г | story is capable of resisting a seismic force = Ω o times the | | |
| | 5 | Weak Story | 5a. Moderate: A < 80% B | | 9 9 | | $_{x}I_{x}$ | | | | lol | \mathbf{x} | | 9 | design force (ASCE 7-05, Section 12.3.3.2). | | |
| | | | 5b. Extreme: A < 65% B | | - | | · | | | | | | | | | | |
| D | 1 . | | 2. Reinforced Concrete Design of | — | | لــــــــــــــــــــــــــــــــــــــ | | | | . /- | لبيا | | | | | l | |



Living with earthquake

Earthquakes and the world

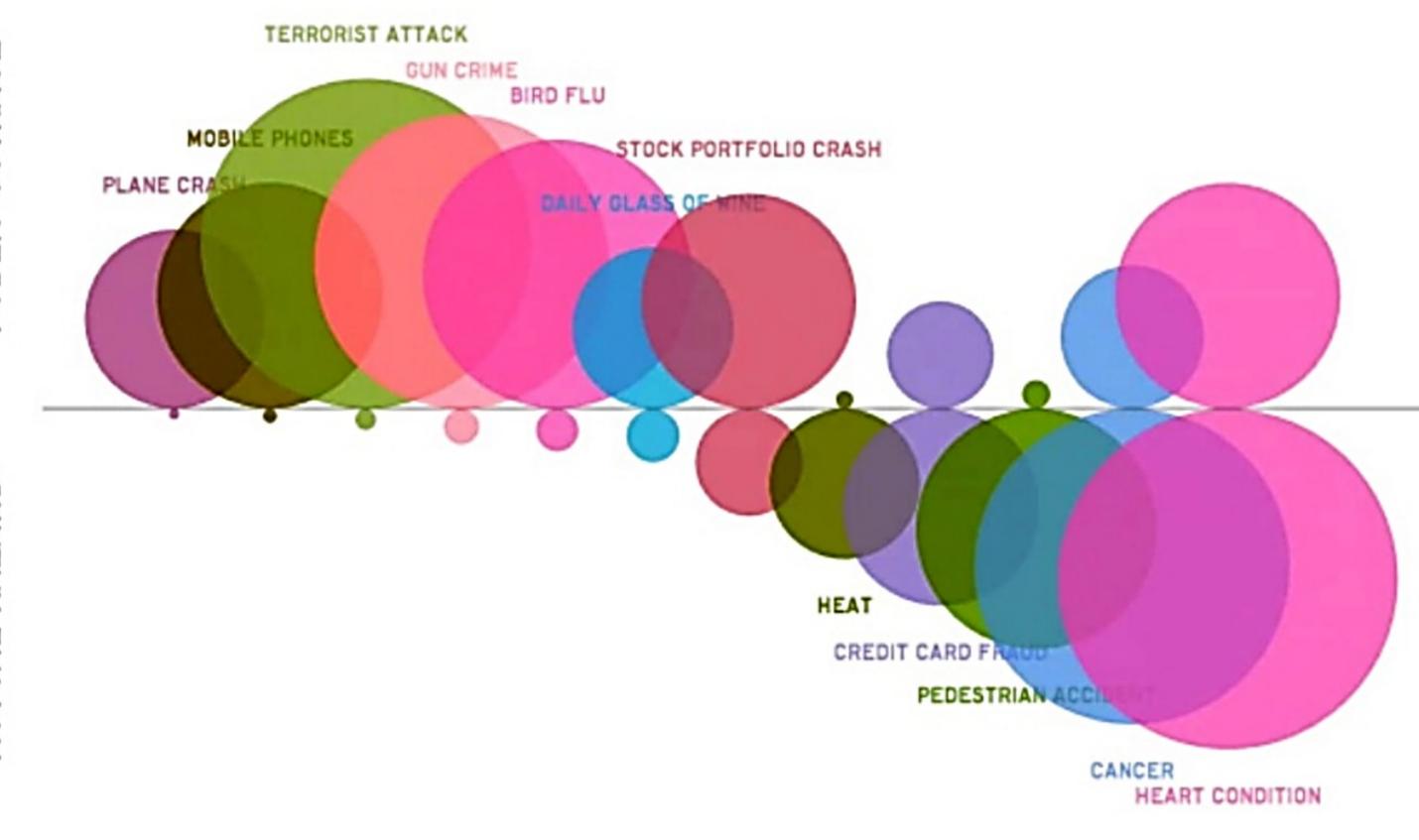
- Earth is 4.6 billion years old
- Oldest human identified 100,000 years ago
- Human civilization started 10,000 years ago



Risk



Risk Perception





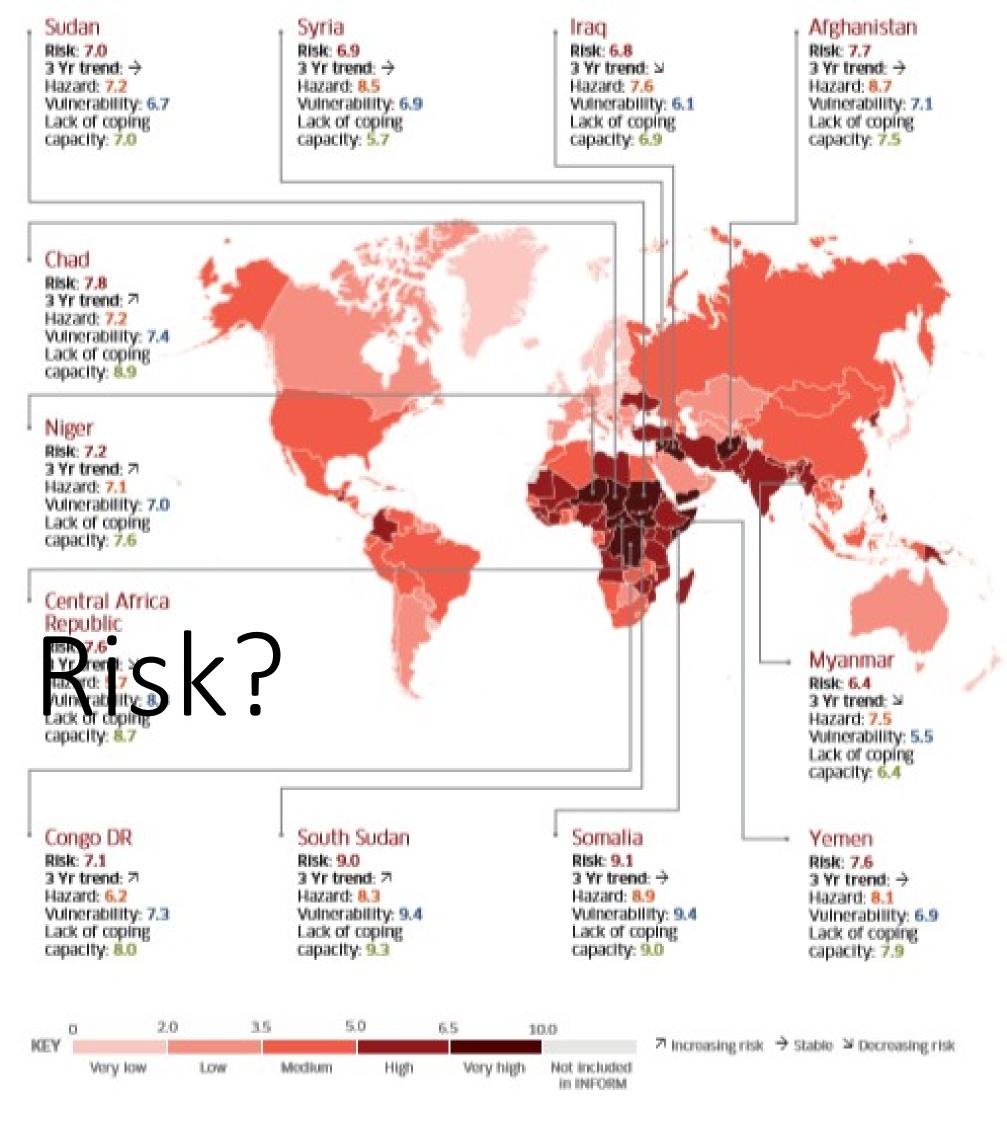
Risk

| | Pr | obability of Dying | g in any O | ne Year |
|---------------|-------|-------------------------------|----------------|--------------|
| | | Cause | Risk | Consequences |
| | | Motor vehicle accident | 1 in 100 | Low |
| | | Smoking 10 cigarettes a day | 1 in 200 | Low |
| | | All natural causes, age 40 | 1 in 850 | Low |
| | Any | kind of violence or poisoning | 1 in 3300 | Low |
| | | Influenza | 1 in 5000 | Medium |
| | | Leukemia | 1 in 12,500 | Low |
| | | Asteroid or comet impact | 1 in 20,000 | Very High |
| | | Playing field sports | 1 in 25,000 | Low |
| | | Accident at home | 1 in 26,000 | Low |
| Low Probabil | ity – | Accident at work | 1 in 43,500 | Low |
| High Conseque | ences | Nuclear War | 1 in 50,000 | Very High |
| | | Tornado | 1 in 60,000 | Medium |
| | | Floods | 1 in 100,000 | High |
| | | Earthquake | 1 in 2,000,000 | High |

Hit by lightning 1 in 10,000,000

Low





What is



Exposure

Exposure

Risk

Vulnerability

Hazard

Risk

Risk = Hazard x Exposure x Vulnerability

Vulnerability

Hazard

Risk

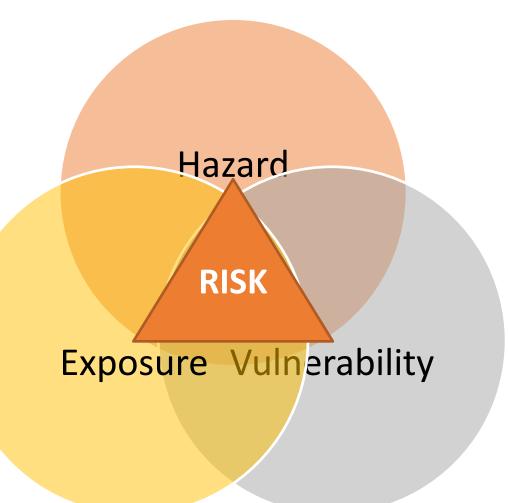
Hazard

Exposure

Vulnerability



Element of Risk





Risk = Hazard x Exposure x Vulnerability

Vulnerability



No Exposure





Risk = Hazard x Exposure x Vulnerability

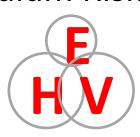
Vulnerability



No Exposure



Medium Exposure Medium Risk





Risk = Hazard x Exposure x Vulnerability

Vulnerability



Medium Exposure Medium Risk



No Exposure





High Exposure High Risk





Risk = Hazard x Exposure x Vulnerability

Vulnerability

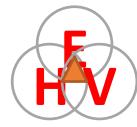


Medium Exposure Medium Risk





High Exposure High Risk





Disaster



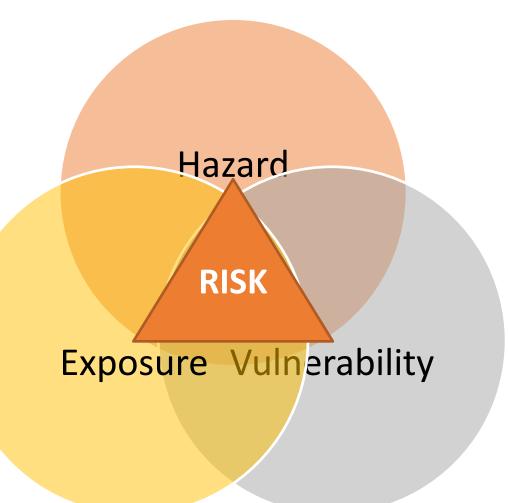


No Exposure





Element of Risk

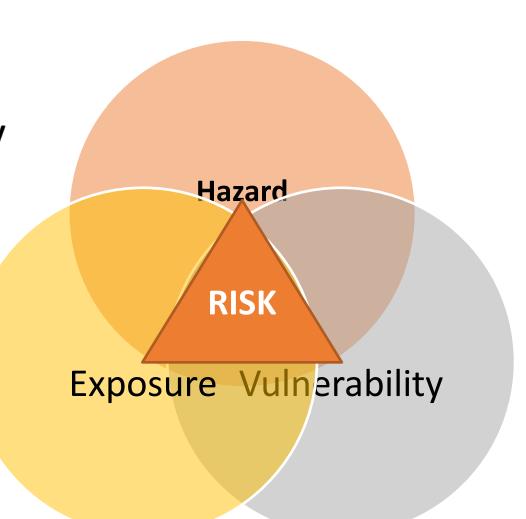




Hazard

- Act of Nature
- Can not be controlled





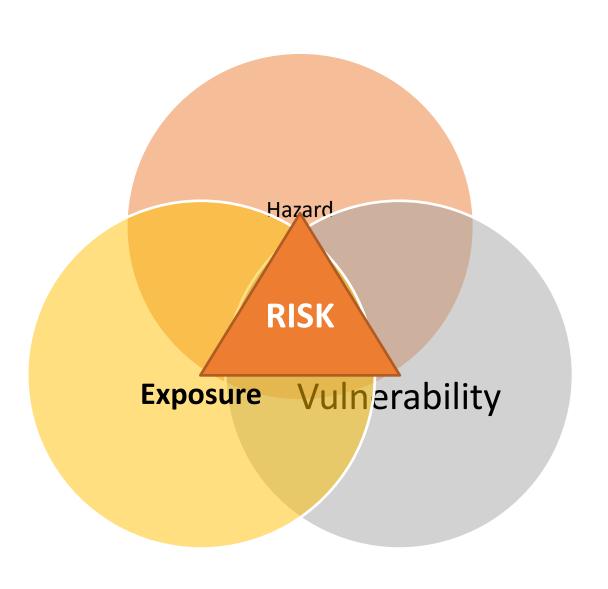


Exposure

- Act of Human
- Urbanization: Global Trend



Risk = Hazard x Exposure x

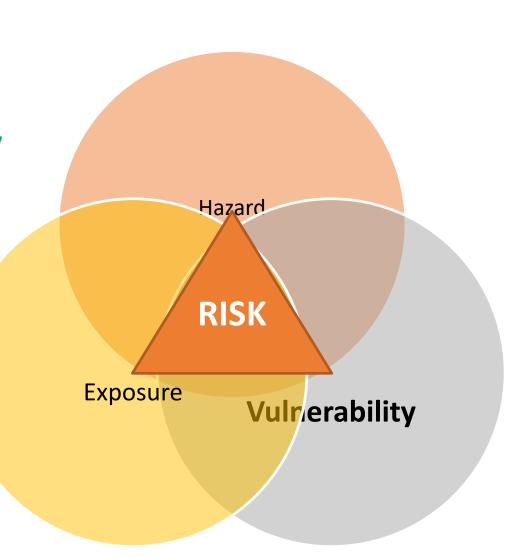




Vulnerability

- Act of Human
- Reduce Vulnerability
- Increase Knowledge
- Enhance Resilience







Risk Reduction

Reduce Vulnerability

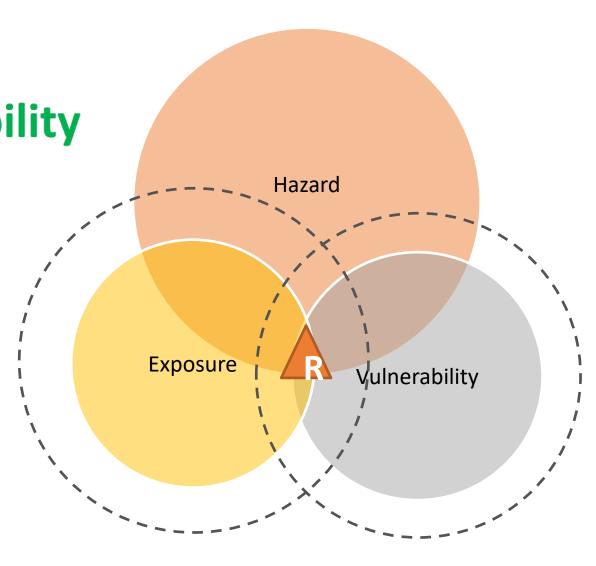
- Increase Knowledge
- Enhance Resilience

Risk = Hazard x Exposure x Vulnerability



Reduce Exposure

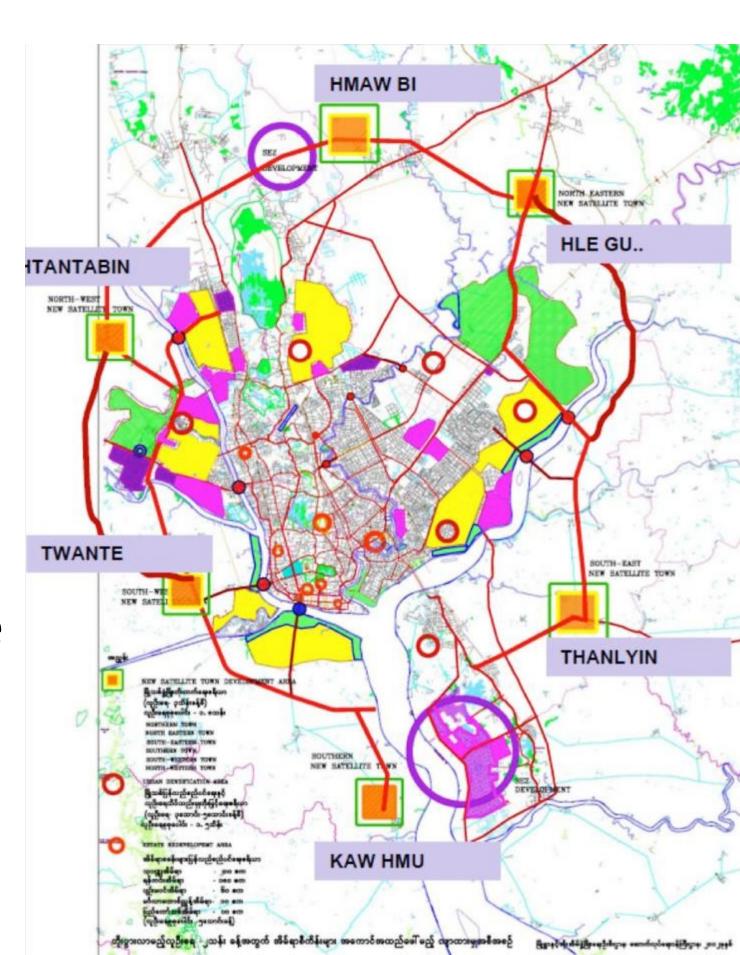
- Control Urbanization
- Mitigate from High Risk Areas





Reduce Exposure

- City Planning
- Control Urbanization
- Mitigate from High Risk areas
- Invest more in Infrastructure and Life Lines

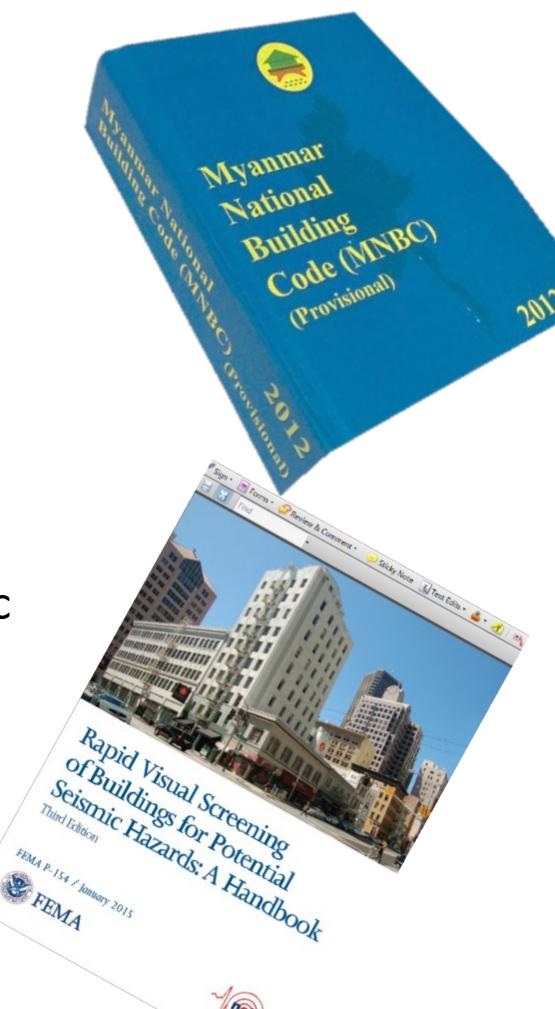




Reducing Vulnerability

- New Building
 - Engineered Building → Control by Building Code
 - None Engineered Building → Public
 Education

- Existing Building
 - Visual Inspection





Thanks